

10/15/19 Pre-Proposal Meeting for Preliminary Engineering for Mann Valley

Question 1: For wetland delineations, should the consultant review the entire concept area or just the City property on Exhibit B?

The consultant should review wetland delineations for the entire area highlighted in Exhibit B.

Question 2: Does the City want a basic ALTA survey or specific items from Table A?

The City would like the consultant to complete Items 1, 3, 13, and 18 from Table A in addition to the minimum requirements of the ALTA survey. Exhibit E has been revised for the ALTA survey (see attached). The City will obtain the title commitments for the ALTA survey after a consultant agreement is in place.

Question 3: What are the City's expectations for 30 percent design?

Would like to see overall site utility layout in plan view and overall site grading in plan view to include street and stormwater areas. Also include preliminary plan and profile sheets showing utilities and streets. 30 percent does not include cross sections.

Question 4: What are the City's expectations for 30 percent design of the lift station?

Consultant shall provide preliminary lift station design and evaluation sufficient to identify a location and cost for the lift station. The selected consultant will be provided the ultimate flow rates for lift station design.

Question 5: What information should be provided for stormwater?

Preliminary stormwater modeling should be provided to determine sizes of ponding and area to be dedicated to stormwater management. 100 percent of the stormwater for the site will be handled with regional stormwater facilities. With 30 percent design, looking for the cost associated with stormwater management.

Question 6: When should consultants focus on the wetland delineation work?

Spring 2020.

Question 7: How much is the City interested in looking at the lots in the steep slope areas to see how developable they are?

The intent is to not do any mass site grading and other than for the roads and storm water management facilities, 30 percent does not include a grading plan. Lot layout shall be limited to the large developable areas that are not encumbered by stormwater, roadways, or steep slopes.

Question 8: What information is needed for water modeling?

The City works with a consultant for water modeling of the entire City. Water modeling has been completed for this area and any pertinent information will be provided to the selected consultant. The City will specify the size of all water mains.

Question 9: Will there have to be small utility coordination?

Coordination with small utilities is not necessary for preliminary design. However, small utilities along Radio Road and Division Street should be identified to aid in decision making for proposed water and sewer routing. The consultant shall identify private utility conflicts for proposed water and sewer improvements.

Question 10: Will the consultant need to provide a rough estimate for street lighting costs?

The City will provide an allowance amount for the consultant to include in their estimate for street lighting.

Question 11: Will the consultant need to provide a rough estimate for landscape design?

The City will provide an allowance amount for the consultant to include in their estimate for landscaping.

Question 12: Does the 30 percent plan include the water loop?

The 30 percent plans should include the water main extension along Radio Road to the future water main connection at Paulson Road. Using County contour surface data is sufficient for 30 percent design.

Question 13: Does the consultant need to identify a site for the PRV at the intersection of Radio Road and Powell Ave?

The City and the consultant will work together to identify a location and cost for the PRV.

Question 14: Is there a desire by the City to get the project finished before August 1, 2020?

Special consideration will be given to consultants that can minimally provide *cost estimates* by May 15, 2020 in order to meet the City's CIP schedule. Other deliverables can be provided after that date.

Question 15: What level of detail is the City looking for the traffic and transportation impacts as well as the intersections that will be required for Road A and Road B?

The consultant shall design an intersection with a left turn lane from Powell Ave. and an intersection with a right turn lane from CTH M. The City will provide a traffic count and will work with the selected consultant to determine timing of the count.

Question 16: Is the City looking for hydraulic sizing for the water crossing of Road A? Does 30 percent include coordination with FEMA?

See the attached Mann Valley Hydraulic Model. 30 percent does not involve coordination with FEMA.

Question 17: How can consultants obtain the latest GIS information?

Contact City Engineer Crystal Raleigh for shapefiles of the corporate limits and water/sewer infrastructure in DeSanctis Park. The North Loop Extension water plan is attached.

MANN VALLEY – PRELIMINARY ENGINEERING
CITY OF RIVER FALLS, WI

PRE-PROPOSAL MEETING
TUESDAY, OCTOBER 15, 9:00 AM
RIVER FALLS CITY HALL – TRAINING ROOM

ATTENDEE LIST

NAME	ORGANIZATION	PHONE#	EMAIL ADDRESS
John Parrotti	SEH	715-861-4871	jparrotti@sehinc.com
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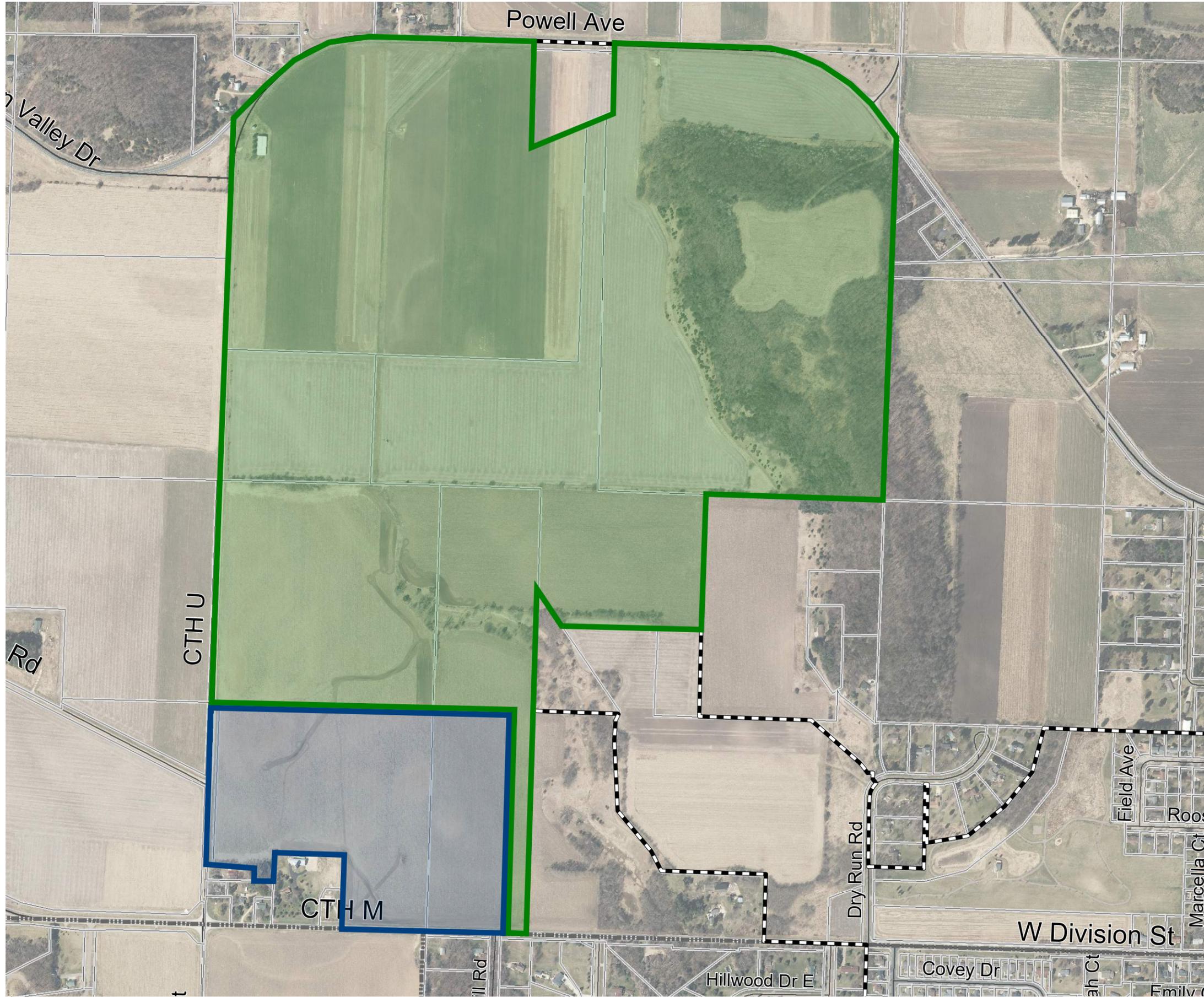


EXHIBIT E

ALTA / NSPS SURVEY LIMITS

MANN VALLEY PRELIMINARY DESIGN

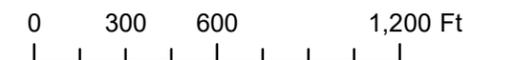
LEGEND

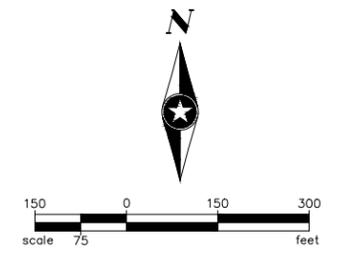


ALTA / NSPS Survey Boundary
for City Owned Property



Alternate ALTA / NSPS Survey
Boundary for Adjacent Property

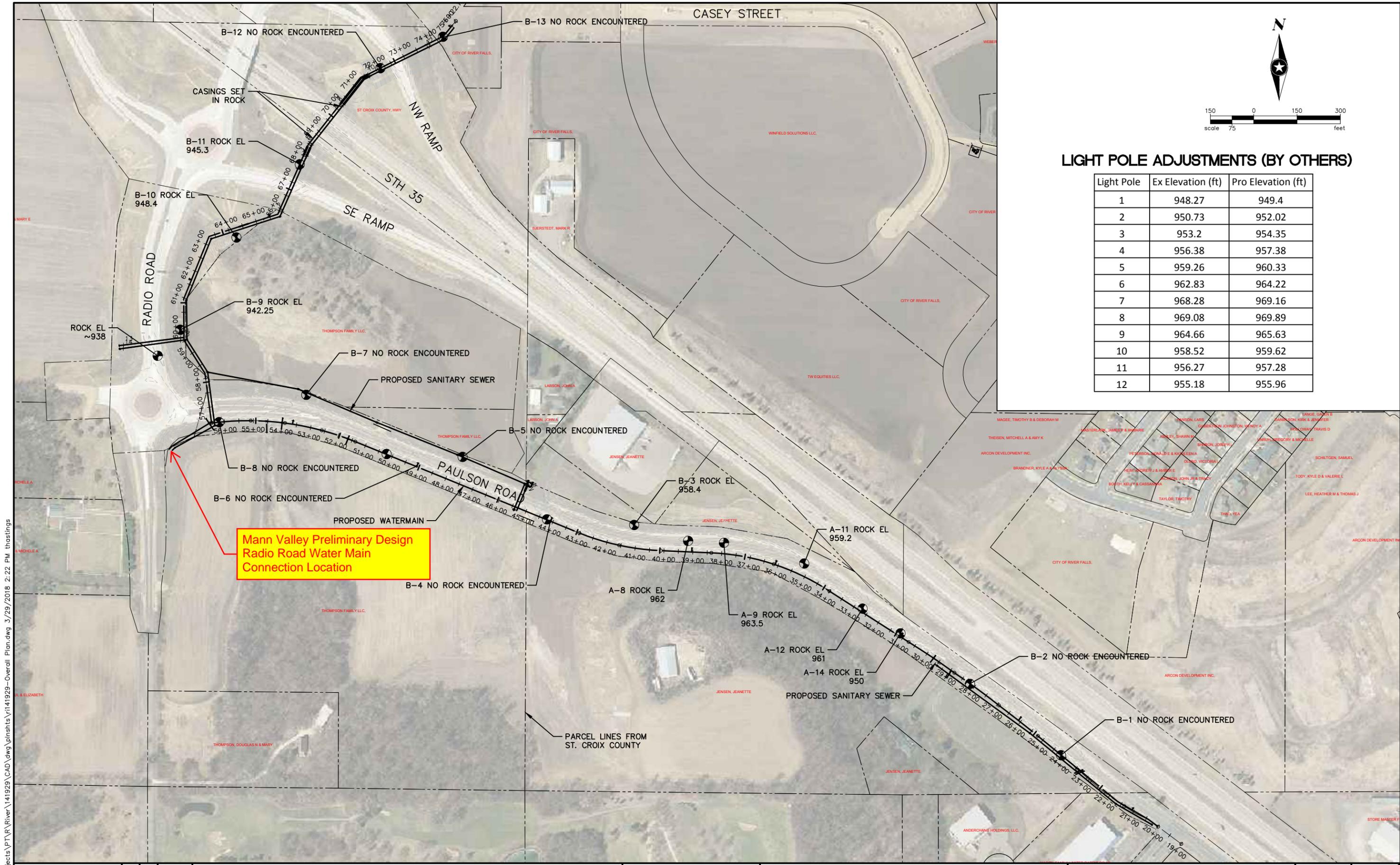




LIGHT POLE ADJUSTMENTS (BY OTHERS)

Light Pole	Ex Elevation (ft)	Pro Elevation (ft)
1	948.27	949.4
2	950.73	952.02
3	953.2	954.35
4	956.38	957.38
5	959.26	960.33
6	962.83	964.22
7	968.28	969.16
8	969.08	969.89
9	964.66	965.63
10	958.52	959.62
11	956.27	957.28
12	955.18	955.96

**Mann Valley Preliminary Design
Radio Road Water Main
Connection Location**



\\SEHNR\Projects\PTV\River\141929\Overall_Plan.dwg 3/29/2018 2:22 PM thostings
 \\SEHNR\Projects\PTV\River\141929\CAD\dwg\planshts\141929\Overall_Plan.dwg 3/29/2018 2:22 PM thostings

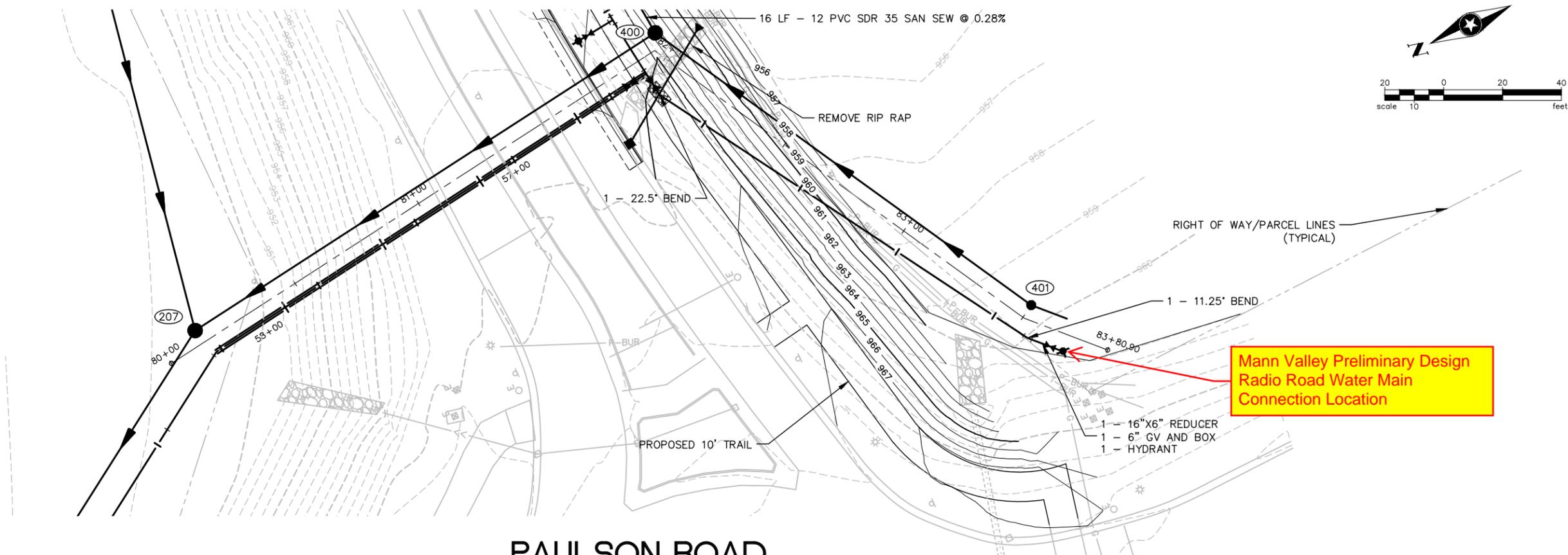
DRAWN BY: TJH
 DESIGNER: EDH
 CHECKED BY: DFS
 DESIGN TEAM

NO.	BY	DATE	REVISIONS

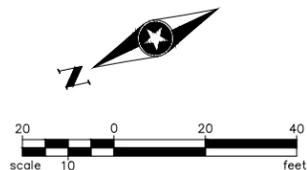

 PHONE: 715.246.9906
 156 HIGH STREET
 SUITE 300
 NEW RICHMOND, WI 54017
 www.sehinc.com

**NORTH LOOP EXTENSION
RIVER FALLS, WISCONSIN**

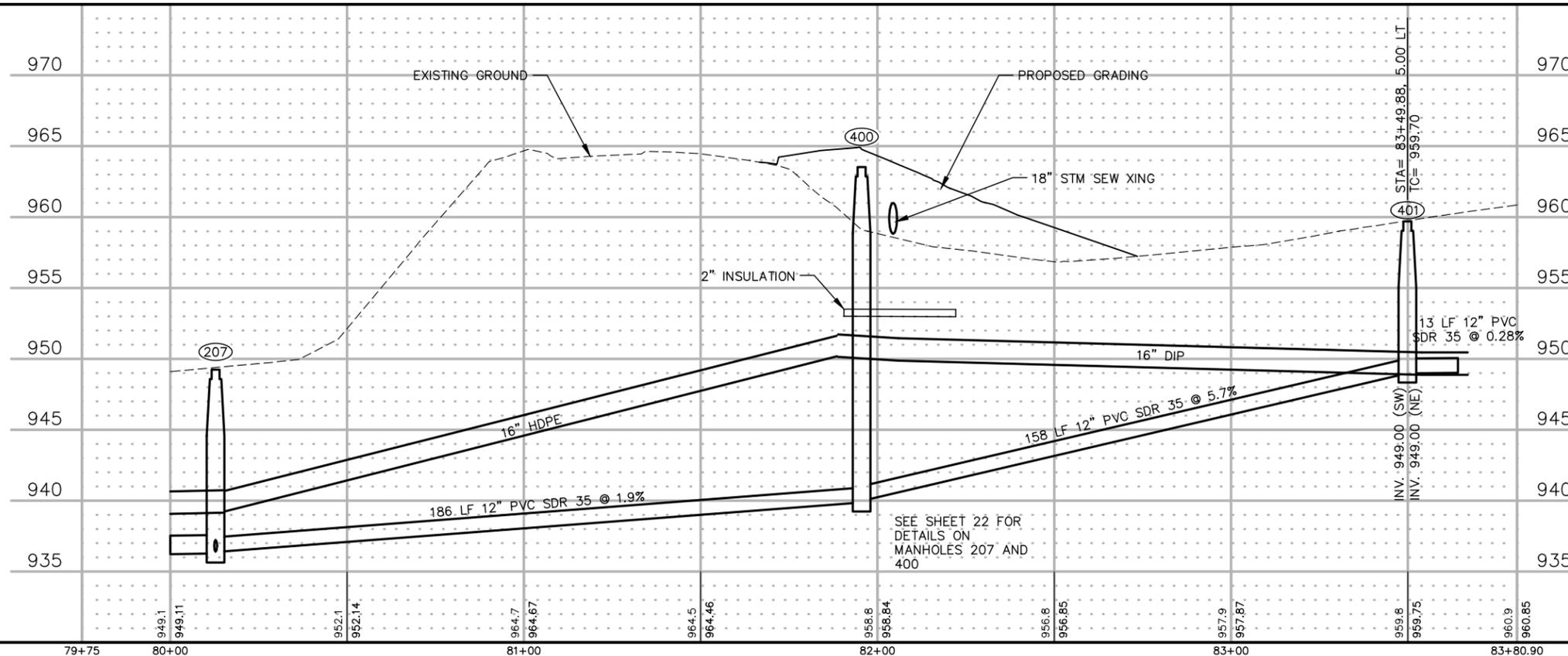
OVERALL PLAN
 FILE NO. RIVER-141929
 DATE 3/29/2018



Mann Valley Preliminary Design
Radio Road Water Main
Connection Location



PAULSON ROAD



\\SEHNF\Projects\PTA\F\River\141929\CAD\dwg\planshts\Paulson Road Xing PP.dwg 3/29/2018 4:35 PM thastings

DRAWN BY: TJH
DESIGNER: EDH
CHECKED BY: DFS

DESIGN TEAM	NO.	BY	DATE	REVISIONS

SEH
PHONE: 715.246.9906
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**NORTH LOOP EXTENSION
RIVER FALLS, WISCONSIN**

**PAULSON ROAD CROSSING
PLAN AND PROFILE**

FILE NO.
RIVER-141929
DATE
3/29/2018

33
55

Mann Valley Hydraulic Model Result

Information in this Section:

QA/QC Checklist

CHECK-2 (printouts)

BOSS-HEC-2 (printouts)

CROSS-SECTIONS (plots)





QA / QC Checklist for HEC-2 Analysis

Job Name: RIVER FALLS FIS
SEH JOB# A-RIVER9601
FILE: 17.00

STREAM: <u>WANN VALLEY</u>	BRANCH: <u>MAIN</u>
FILENAME: <u>RASMANNV</u>	METAJOB No.: <u>1</u>
By: <u>MEB</u>	Checked By: _____
Date: <u>9/23/97</u>	Date: <u>1/1</u>

Model:

Version: RMS HEC-2 Boss™ ver 3.0

Meets the Following Program Assumptions and Limitations (Check Boxes):

YES / NO (Explain: _____)

- 1. Flow is Steady (Discharge Rate Constant During Study Time Interval)
- 2. Flow is Gradually Varied (Depth Changes Gradually over a Short Distance)
- 3. Flow is 1-Dimensional (Velocity Component in directions other than the direction of flow unaccounted for)
- 4. Small River Channel Slopes (less than 1:10)
- 5. Flow is Fully Turbulent and Boundary Friction can be Described by Manning's or Chezy's
- 6. Flow Boundary is Rigid
- 7. Water is Homogenous and Incompressible
- 8. Effects of Transverse Velocity Distributions can be Approximated Using Alpha and Channel Subdivision

General Input Data:

Computation Limits from: 8638
to: 12460
Study Limits (If Different than Computation Limits) 9576 → 12460 (MM → upstream)
Starting WS Elevation = Normal Depth (preferred by FEMA), Slope-Area (preferred by FEMA) or Other (with Comment)
Sensitivity Performed for Initial WS Elevation Yes / No ~ several x-sections between bottom & study limits
Friction Loss Computation Method / Comment Automatic
Study Discharge Rates (cfs) Source: Rocky KEEN 3500
10-YR _____ 50-YR _____ 100-YR 800/1770/2000 500-YR 2800
Other: _____ Compare w/ FIS: _____
Cross-Section of Change in Q N/A (would change d.s. of C.T.H. map)
Reason for Change in Q (tributary, etc.)
Storage Reasonably Taken Into Account HydroCAD™ (H20 method used by Rocky) ~ Yes

Cross-Sections:

Survey By: _____ Previous FIS: _____
Source of Ground Geometry From City Map: (Scale :) Contour Interval: 2' (Electronic) Paper
Source of Geometry Below Water Surface ~ Rocky Keen site Survey By: SEH Previous FIS: _____
Numbering System 2 distance from Kinnickiniz River
Overbank Stationing / Channel Lengths Checked -Yes, MSL
Spacing: OK ~ generally < 500 ±
Spacing Guidelines:
1. Where change in xs area or shape, roughness, and bed slope
2. Normal to flow lines
3. Significant changes in conveyance (width, depth, n values)
4. Above, below and within bridges
5. Head and tail of levees
6. All control sections
7. Below a confluence on main stream and above confluence on the tributary
8. Generally more cross-sections needed in urban areas, steeper slopes, and smaller streams
**** NOT LESS THAN 1/2 MILE - SUGGEST 500 FT SPACING ****
Left Overbank Description: brush / crops Left Overbank Manning's n varies 0.07 → 0.1
Channel Description: small, relatively clean Channel Manning's n varies 0.04
Right Overbank Description: brush / crops Right Overbank Manning's n varies 0.07 → 0.1
Sources / Validation of Manning's n Values Rtk field visit, photos, USBS Documents/Methodology
Manning's Values Compare with Original FIS N/A



QA / QC Checklist for HEC-2 Analysis

Job Name: RIVER FALLS FIS
SEH JOB# A-RIVER9601
FILE: 17.00

STREAM: <u>MANN VALLEY</u>	BRANCH: <u>Main</u>
FILENAME: <u>RASMANN</u>	METAJOB No.: <u>1</u>
By: <u>MER</u>	Checked By: _____
Date: <u>3/23/97</u>	Date: <u>1/1</u>

Bridge Hydraulics:

Name of Bridge C.T.H M M

General Description: 42' 28" I Girder

Modeling Technique (Valley / Normal Bridge / Special Bridge) Special Bridge

Source of Geometry Information / Datum County Provided WSPRO Information / Unknown

Alignment / Skew Comments -No adjustments made

Minimum Road / Low Member Elevations 880 / 876.55

Expansion / Contraction Coefficients Used 0.5/0.3

Pier Width / Shape Coefficient N/A

Bridge Total Loss Coefficient (Generally 1.56) 1.56

Encroachments Used (Upstream / Downstream) Downstream (4:1) / Bridge ~ width ~ 40' / upstream ~ not needed due to taper

Compatibility w/ Bridge Design Hydraulic Info - Other Used Info from bridge design WSPRO

Meets Roadway Authority Protection Requirements Yes

Name of Bridge _____

General Description: _____

Modeling Technique (Valley / Normal Bridge / Special Bridge) _____

Source of Geometry Information / Datum _____

Alignment / Skew Comments _____

Minimum Road / Low Member Elevations _____

Expansion / Contraction Coefficients Used _____

Pier Width / Shape Coefficient _____

Bridge Total Loss Coefficient (Generally 1.56) _____

Encroachments Used (Upstream / Downstream) _____

Compatibility w/ Bridge Design Hydraulic Info - Other _____

Meets Roadway Authority Protection Requirements _____

Name of Bridge _____

General Description: _____

Modeling Technique (Valley / Normal Bridge / Special Bridge) _____

Source of Geometry Information / Datum _____

Alignment / Skew Comments _____

Minimum Road / Low Member Elevations _____

Expansion / Contraction Coefficients Used _____

Pier Width / Shape Coefficient _____

Bridge Total Loss Coefficient (Generally 1.56) _____

Encroachments Used (Upstream / Downstream) _____

Compatibility w/ Bridge Design Hydraulic Info - Other _____

Meets Roadway Authority Protection Requirements _____



QA / QC Checklist for HEC-2 Analysis

Job Name: RIVER HILLS FIS
SEH JOB# A-RIVER 9601

STBAM: MAVIN VALLEY FILE: 17.00
 EFILENAME: KRSMANUV BRANCH: M/FIN
 By: MEB METAJOB No.: 1
 Date: 3/23/97 Checked By: _____
 Date: 1/1

Other (Dams etc.)

Description: N/A
 Modeling Method: _____
 Comments: _____

Calibration Data Available: _____
 Comments: None Available - however bridge design CWSER = 870.04, Q = 700 cfs

Conveyance Range Messages Can Occur if (check if problem / reasonable):
 1. Large change in flow distribution from channel to overbanks or reverse
 2. Large change in top widths
 3. Additional flow from tributaries
 4. Large change in capacity (cross-section, roughness, slope)
 Comments: @ Bridge, downstream of study limits - ok.

Comments Regarding Other Areas of Instability or Locations of Interest
NONE

Energy Grade Line Slope should not decrease by more than 50% or increase more than 100% between sections (if it does need more cross-sections to define reach)
Not a concern due to XS density

Critical Depth Occurrences (Run Supercritical Option to Check if Composite is Needed)
 Comments (Note: FEMA states, if supercritical flow, use critical water surface elevation):
Downstream of bridge beyond study limits - ok.

Check Velocities if Reasonable - Should X-Section Geometry Be Altered to Account for Scour?
ok

Bridge Hydraulics
 Check if flow depths, widths, etc. seem reasonable / Does it meet roadway authority's protection requirements
 Check if calculated flow area - Input flow area for special bridge option for full flow scenarios
 Comments: Looks good

Comparison of Flood Profiles with Calibration Data
 Comments: ok

Comparison of Flood Profiles with Original FIS
 Comments: N/A

Comments on Reasonableness of Profile - Conclusions:
Profile Appears Reasonable.

CHECK-2 Program: NT Module
Manning's n Value and Transition Loss Coefficient Review

Report File = S:\WATER\ANONTECH\ROCKY\MANNV.NT2
 Date: 11/17/97
 Time: 7:20:55 PM

ECNO	XNL	XNR	XNCH	CCHV	CEHV	Structure
8638	0.07	0.07	0.04	0.1	0.3	
8989	0.07	0.07	0.04	0.1	0.3	
9256	0.07	0.07	0.04	0.1	0.3	
9536	0.07	0.07	0.04	0.3	0.5	
9576	0.07	0.07	0.04	0.3	0.5	SB+X2
9724	0.1	0.1	0.04	0.3	0.5	
10125	0.1	0.1	0.04	0.1	0.3	
10492	0.07	0.07	0.04	0.1	0.3	
10912	0.07	0.07	0.04	0.1	0.3	
11619	0.07	0.07	0.04	0.1	0.3	
12068	0.07	0.07	0.04	0.1	0.3	
12460	0.07	0.07	0.04	0.1	0.3	

-----Summary of Statistics-----

		Minimum	Maximum
Left Overbank	N Value:	0.07	0.1
Right Overbank	N Value:	0.07	0.1
Channel	N Value:	0.04	0.04
Contraction	Coefficient:	0.1	0.3
Expansion	Coefficient:	0.3	0.5

-----Roughness Coefficient Check-----

-----NC and NH records Check-----

-----Transition Coefficient check-----

-----Roughness Coefficient Check at Structures-----

-----Special Notes and Messages check-----

-End Program---

CHECK-2 Program, XSEC Module
Cross Section Location and Alignment Review

Report File = S:\WATER\ANONTECH\ROCKY\MANNV.XS2
Date: 11/17/97
Time: 7:21:13 PM

SECNO	Xlob	Xrob	Xlch	CHWID	Topwid	Structure Flow Type
8638	0.00	0.00	0.00	3.89	97.72	
8989	350.86	348.96	351.46	3.96	57.66	C
9256	231.56	341.96	266.29	22.22	152.08	
9536	310.97	217.22	280.72	38.17	35.93	
9576	40.00	40.00	40.00	36.63	39.99	SB+X2
9724	172.68	150.30	148.29	13.44	195.50	
10125	384.03	462.31	400.84	4.27	230.15	
10492	370.23	360.72	366.80	4.66	110.32	
10912	399.80	453.69	420.48	4.38	183.75	
11619	724.54	685.20	706.87	3.81	75.85	
12068	470.13	435.88	448.72	3.98	110.85	
12460	349.37	411.55	392.45	4.47	158.85	

S IF 04 D = divided flow
XS IF 05 E = extended cross section
XS IF 06 C = critical depth
XS IF 07 S = X5 record

-----Distance Check-----

SECNO: 9724
XS DT 01 Both right and left overbank distances are longer than the channel distance.

---Channel Width Check---

SECNO: 9256
XS CW 02 Channel width changes by a factor of two when compared to the the downstream cross section.

SECNO: 10125
XS CW 02 Channel width changes by a factor of two when compared to the the downstream cross section.

---Spacing Check---

---Ineffective Flow Area---

---Location Check---

---Discharge Check---

XS DC 02 Constant discharge used for the entire profile.

---Starting WSEL Check---

XS SW 02 Known water-surface elevation of 865.43 is specified on the first J1 record.

--End Program---

Report File = S:\WATER\ANONTECH\ROCKY\MANNV.BR2
 Date: 11/17/97
 Time: 7:22:08 PM

SECNO	MXLCEL	MNREL	EG	CWSEL	ELMIN	Structure
8638	-----	-----	866.52	865.43	856.9	
8989	-----	-----	875.47	873.63	864	
9256	-----	-----	876.06	875.96	864	
9536	-----	-----	877.4	875.38	866	
9576	876.55	880.3	878.01	876.46	866	SB+X2
9724	-----	-----	878.68	878.57	865.7	
10125	-----	-----	878.93	878.89	867.2	
10492	-----	-----	879.32	879.12	869.4	
10912	-----	-----	880.35	880.19	872.6	
11619	-----	-----	884.92	883.74	876	
12068	-----	-----	888.03	887.76	878.8	
12460	-----	-----	889.65	889.36	882	

ECNO: 9576
 Type Structure: Special Bridge
 Type of Flow: Special Bridge Reverts to Normal Bridge Method

Section	SECNO	XLCH	CWSEL	EG	TOPWID	CHWID	X3 Left	X3 Right
1	9256	266.29	875.96	876.06	152.08	22.22		
2	9536	280.72	875.38	877.4	35.93	38.17	0	0
3	9576	40	876.46	878.01	39.99	36.63	0	0
4	9724	148.29	878.57	878.68	195.5	13.44		
5	10125	400.84	878.89	878.93	230.15	4.27		

EGLWC: 877.78 QLOW: 2000 WRLEN: 0
 EGPRS: 876.46 QPR: 0
 BR.WS: 0 QWEIR: 0
 BR.VEL: 0 Q: 2000
 MNLRDEL: 880.3 MXLCEL: 876.55 ELLC: 876.55
 MNRDEL: 880.3 MNRDEL: 880.3 ELTRD: 880.3
 XK: 0 XKOR: 1.56 COFQ: 2.63
 RDELEN: 40 BWC: 15 BWP: 0
 BAREA: 300 SS: 1 ELCHU: 0 ELCHD: 0

---BT records Check---

SECNO: 9576
 BR BT 01 Left channel bank station is not specified within the BT data.
 GR and BT data may not be aligned properly.

SECNO: 9576
 BR BT 02 Right channel bank station is not specified within the BT data.
 GR and BT data may not be aligned properly.

---Location check---

---X2 record check---

---X3 record check---

SECNO: 9536
 BR X3 16 Section 2; Area below 880 and beyond left encroachment station
 1263.51 will be not be considered in the backwater computations.

SECNO: 9536
 BR X3 17 Section 2; Area below 880 and beyond right encroachment station
 1307.99 will be not be considered in the backwater computations.

SECNO: 9576
 BR X3 16 Section 3; Area below 880 and beyond left encroachment station
 1338.01 will be not be considered in the backwater computations.

SECNO: 9576
BR X3 17 Section 3; Area below 880 and beyond right encroachment station
1378 will be not be considered in the backwater computations.

--Low Flow Check---

SECNO: 9576
BR LF 09 ELCHU value is not given on SB record. ELMIN at section 2
will be used as ELCHU.
269.5521 300

BR LF 08 Trapezoidal Area is 269.5521 ; BAREA is 300
The difference between the two should not be more than 10%.

SECNO: 9576
BR LF 12 Special Bridge, revert back to normal bridge routine.
The bridge should be remodeled as normal bridge routine.
If the structure is a box or circular culvert, please use
special culvert option.

SECNO: 9576
BR PF 04 Computed XKOR of 1.71 is different from the XKOR of 1.56 on field 2 of SB record.
CHECK2 uses an n value of 0.03 , which is less than the channel n value
at section 3 by 0.01. The data used in the XKOR computation are:
n = 0.03 ; A = 269.55; P = 80.94; R = 3.33; L = 40 ; Kf = 0.21; Ke = 0.50
If the data is correct, the XKOR in the input file should be changed.
Otherwise, please give explanation on the data used to compute the XKOR in
the report file.
If the structure is a box or circular culvert, use special culvert routine.

---Message Check---

--End Program---

BOSS HEC-2 version 3.00
PROJECT TITLE : RIVER FALLS FLOOD INSURANCE STUDY
PROJECT NUMBER : A-RIVER9601.00

PAGE 1
11/12/1997

=====
B O S S H E C - 2 (tm)
=====

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Version : 3.00
Serial Number : 0020213.250

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PROGRAM ORIGIN :

Boss Hec-2 (tm) is an enhanced version of the U.S. Army Corps of Engineers Hydrologic Engineering Center HEC-2 program for water-surface profile computations. Program based upon the September 1990 version, updated on August 1991.

DISCLAIMER :

Boss Hec-2 (tm) is a complex program which requires engineering expertise to use correctly. Boss Corporation assumes absolutely no responsibility for the correct use of this program. All results obtained should be carefully examined by an experienced professional engineer to determine if they are reasonable and accurate.

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PROJECT DESCRIPTION :

PROJECT TITLE : RIVER FALLS FLOOD INSURANCE STUDY
PROJECT NUMBER : A-RIVER9601.00
DESCRIPTION : MANN VALLEY
ENGINEER : M RYE/ R KEEHN
DATE OF RUN : 11/12/1997
TIME OF RUN : 6:36 am

T1 A-RIVER9601.00
 T2 RIVER FALLS FLOOD INSURANCE STUDY
 T3 MANN VALLEY
 T4 File Name is MANNV.HC2 (BOSS HEC-2)
 T4 Work was completed by Short Elliott Hendrickson, Inc., St Paul, MN
 T4 No previous FIS information had been prepared for this reach.

JOB PARAMETERS :

J1	ICHECK	INQ	NINV	IDIR	STRT	METRIC	HVINS	Q	WSEL	FQ
0		2	0	0	0.0	0	0.0	850.0	863.38	
J2	NPROF	IPLOT	PRFVS	XSECV	XSECH	FN	ALLDC	IBW	CHNIM	ITRACE
1		1	0.0	0.0	0.0	0.0	-1.0	-6.0	0.0	15

USER-DEFINED SUMMARY TABLES (J3) :

150	38	43	1	51	3	61	4	58
2	38	43	1	52	8	39	14	13
15	38	1	13	55	16	14	26	17
15	56	18						

REQUESTED CROSS-SECTION PRINTOUTS (J5) :

-10 -10
 FRICTION LOSS EQUATION OPTION (J6) :

J6	IHLEQ	ICOPY	SUBDIV	STRTDS	RMILE
1	0	0.0			

NC	0.07	0.07	0.04	0.1	0.3
M.V. @ 8638' N/KINN					
M.V. @ 8638' N/KINN					

X1	8638	14	1043.73	1047.62	0.0	0.0	1.0	0.0
GR	870.0	1000.0	868.0	1003.88	866.0	864.0	1015.08	862.0
GR	860.0	1034.46	858.9	1043.73	856.9	858.9	1047.62	1020.99
GR	862.0	1061.77	864.0	1073.39	866.0	868.0	1139.72	860.0
NC	0.07	0.07	0.04	0.0	0.0	351.46	1.0	0.0
X1	8989	16	1037.61	1041.57	350.86			

PROJECT TITLE : RIVER FALLS FLOOD INSURANCE STUDY
PROJECT NUMBER : A-RIVER9601.00

11/12/1997

GR	880.0	1000.0	878.0	1005.67	876.0	1011.4	874.0	1017.12	872.0	1022.85
GR	870.0	1028.58	868.0	1034.18	866.0	1037.61	864.0	1039.73	866.0	1041.57
GR	868.0	1048.34	870.0	1058.07	872.0	1067.87	874.0	1077.63	878.0	1097.55
GR	880.0	1107.53								
X1	9256	19	1094.53	1116.75	231.56	341.96	266.29	1.0	0.0	
X3	0	0.0	0.0	1023.92	880.0	1176.0	880.0	0.0	0.0	
GR	880.0	1000.0	878.0	1009.03	876.0	1015.08	874.0	1023.33	872.0	1044.36
GR	870.0	1065.53	868.0	1086.89	866.99	1094.53	866.0	1102.02	864.0	1104.16
GR	866.0	1106.85	866.99	1116.75	868.0	1126.94	870.0	1156.03	871.33	1176.0
GR	872.0	1186.64	874.0	1205.33	876.0	1219.6	878.0	1233.84		
NC	0.0	0.0	0.0	0.3	0.5					
C.T.H. MM BRIDGE										
C.T.H. MM BRIDGE										
X1	9536	16	1263.51	1301.68	310.97	217.22	280.72	1.0	0.0	
X3	0	0.0	0.0	1263.51	880.0	1307.99	880.0	0.0	0.0	
GR	882.0	1000.0	880.0	1071.55	878.0	1207.55	876.25	1256.38	876.0	1263.51
GR	868.0	1266.36	866.0	1267.04	867.8	1269.32	868.0	1275.4	870.0	1283.51
GR	873.76	1294.78	876.06	1301.68	878.0	1307.5	880.0	1384.35	882.0	1463.98
GR	884.0	1557.89								
NC	0.07	0.07	0.04	0.0	0.0					
SB	0.0	1.56	2.63	40.0	15.0	0.0	300.0	1.0	0.0	0.0
1ST CROSS-SECTION NO. OF ROAD										
1ST CROSS-SECTION NO. OF ROAD										
X1	9576	15	1338.54	1375.17	40.0	40.0	40.0	1.0	0.0	
X2	0.0	0.0	1	876.55	880.3	0.0	0	0.0	0.0	
X3	0	0.0	0.0	1338.01	880.0	1378.0	880.0	0.0	0.0	
GR	884.0	1000.0	882.0	1124.69	880.0	1200.47	878.0	1258.07	876.0	1338.54
GR	868.0	1359.86	866.0	1361.93	868.0	1363.73	870.0	1371.19	872.0	1372.19
GR	874.0	1373.44	876.0	1375.17	876.0	1378.17	880.0	1467.73	882.0	1548.3
NC	0.1	0.1	0.04	0.0	0.0					
X1	9724	19	1092.18	1105.62	172.68	150.3	148.29	1.0	0.0	
GR	884.0	956.66	880.0	1000.0	878.0	1017.58	876.0	1029.29	874.0	1040.33
GR	872.0	1051.92	870.0	1071.55	868.0	1092.18	867.7	1101.21	865.7	1103.39
GR	867.7	1105.62	874.16	1166.54	875.44	1178.59	877.18	1195.02	878.17	1204.3
GR	879.16	1213.6	880.04	1221.87	880.79	1228.94	882.0	1240.39		
NC	0.1	0.1	0.04	0.1	0.3					
X1	10125	21	1112.51	1116.78	384.03	462.31	400.84	1.0	0.0	
GR	892.0	785.08	891.2	845.6	890.78	876.83	890.2	920.21	890.0	935.06
GR	880.0	1000.0	878.0	1011.1	876.0	1022.18	874.0	1035.84	872.0	1051.8
GR	870.0	1091.62	869.2	1112.51	867.2	1114.61	869.2	1116.78	870.0	1143.71
GR	872.0	1167.1	874.0	1200.01	876.0	1220.64	878.0	1232.2	882.12	1251.59
GR	886.0	1269.82								
NC	0.07	0.07	0.04	0.0	0.0					
X1	10492	17	1057.63	1062.29	370.23	360.72	366.8	1.0	0.0	

GR	886.0	974.22	882.37	989.83	880.0	1000.0	878.0	1007.16	876.0	1014.67
GR	874.0	1023.47	872.0	1038.62	871.4	1057.63	869.4	1059.89	871.4	1062.29
GR	872.0	1083.53	874.0	1092.27	876.0	1100.59	878.0	1108.79	880.0	1117.11
GR	883.54	1133.76	888.0	1154.78						
NC	0.07	0.07	0.04	0.0	0.0					
X1	10912	20	1138.67	1143.05	399.8	453.69	420.48	1.0	0.0	
GR	890.0	1000.0	888.0	1009.18	886.0	1018.45	884.0	1027.75	882.0	1036.98
GR	880.0	1047.25	878.0	1065.11	876.0	1095.24	874.6	1138.67	872.6	1140.69
GR	874.6	1143.05	876.0	1181.45	878.0	1206.95	880.0	1228.55	882.0	1245.37
GR	884.0	1262.37	886.0	1278.3	888.0	1289.94	890.0	1332.2	890.0	1357.42
NC	0.07	0.07	0.04	0.0	0.0					
X1	11619	15	1088.14	1091.95	724.54	685.2	706.87	1.0	0.0	
GR	890.0	1000.0	888.0	1022.25	886.0	1032.68	884.0	1043.06	882.0	1054.41
GR	880.0	1061.91	878.0	1088.14	876.0	1089.87	878.0	1091.95	880.0	1100.9
GR	882.0	1110.93	884.0	1121.7	886.0	1133.81	888.0	1145.77	890.0	1165.74
NC	0.07	0.07	0.04	0.0	0.0					
X1	12068	16	1055.59	1059.57	470.13	435.88	448.72	1.0	0.0	
GR	892.0	866.75	890.0	967.8	888.0	1000.0	886.0	1012.52	884.0	1020.08
GR	882.0	1031.11	880.8	1055.59	878.8	1057.79	880.8	1059.57	882.0	1082.63
GR	884.0	1093.33	886.0	1103.29	888.0	1113.65	890.0	1124.53	892.0	1145.5
GR	894.0	1216.73								
NC	0.07	0.07	0.04	0.0	0.0					
X1	12460	16	1087.71	1092.18	349.37	411.55	392.45	1.0	0.0	
GR	892.0	829.24	892.0	872.42	892.0	943.19	890.0	1000.0	888.0	1034.25
GR	886.0	1058.23	884.0	1082.67	884.0	1087.71	882.0	1089.98	884.0	1092.18
GR	884.0	1097.25	886.0	1142.2	888.0	1158.45	890.0	1175.04	892.0	1204.63
GR	894.0	1244.82								

STATUS: Friction loss equation index (IHLEQ) = 1. Therefore friction loss (HL) is calculated as a function of the profile type, which can vary from reach to reach. See documentation for further details.

FLOW DISTRIBUTION :

Station (ft) =	1016.9	1021.0	1034.5	1043.7	1047.6	1050.4
Flow (%) =	.7	17.8	28.4	28.6	28.6	8.1
Area (sq ft) =	2.8	32.1	36.4	21.3	21.3	10.9
Vel (ft/s) =	2.01	4.73	6.62	11.42	6.34	6.34
Depth (ft) =	.69	2.38	3.93	5.48	3.93	3.93
Cross-Section Number (SECNO)						8638.000
Total Discharge (cfs, Q)						850
Computed Water Surface Elevation (ft MSL, CWSEL)						863.38

Station (ft) = 1050.4 1061.8 1069.8
 Flow (%) = 15.0 1.3
 Area (sq ft) = 27.1 5.5
 Vel (ft/s) = 4.71 2.07
 Depth (ft) = 2.38 .69

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)
 Total Discharge (cfs, Q) 8989.000
 Computed Water Surface Elevation (ft MSL, CWSEL) 871.44

Station (ft) = 1024.5 1028.6 1034.2 1037.6 1041.6 1048.3
 Flow (%) = .7 7.6 12.0 39.3 25.4
 Area (sq ft) = 3.0 13.7 15.2 25.5 30.0
 Vel (ft/s) = 2.11 4.76 6.70 13.10 7.18
 Depth (ft) = .72 2.44 4.44 6.44 4.44

Station (ft) = 1048.3 1058.1 1065.1
 Flow (%) = 13.6 1.3
 Area (sq ft) = 23.7 5.1
 Vel (ft/s) = 4.89 2.17
 Depth (ft) = 2.44 .72

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)
 Total Discharge (cfs, Q) 9256.000
 Computed Water Surface Elevation (ft MSL, CWSEL) 873.09

Station (ft) = 1032.8 1044.4 1065.5 1086.9 1094.5 1116.8
 Flow (%) = .2 3.7 11.5 6.9 48.3
 Area (sq ft) = 6.3 44.4 87.5 42.8 153.9
 Vel (ft/s) = .29 .72 1.12 1.38 2.67
 Depth (ft) = .55 2.10 4.10 5.60 6.93

Station (ft) = 1116.8 1126.9 1156.0 1176.0
 Flow (%) = 9.3 15.7 4.3
 Area (sq ft) = 57.1 119.2 48.6
 Vel (ft/s) = 1.38 1.12 .75
 Depth (ft) = 5.60 4.10 2.43

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)
 Total Discharge (cfs, Q) 9536.000
 Computed Water Surface Elevation (ft MSL, CWSEL) 873.80

Station (ft) = 1264.3 1301.7
 Flow (%) = 100.0
 Area (sq ft) = 122.8
 Vel (ft/s) = 6.92
 Depth (ft) = 4.01

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)
 Total Discharge (cfs, Q) 9576.000
 Computed Water Surface Elevation (ft MSL, CWSEL) 874.06

Station (ft) = 1343.7 1375.2
 Flow (%) = 100.0
 Area (sq ft) = 118.5
 Vel (ft/s) = 7.17
 Depth (ft) = 3.98

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)
 Total Discharge (cfs, Q) 9724.000
 Computed Water Surface Elevation (ft MSL, CWSEL) 875.17

Station (ft) = 1033.9 1040.3 1051.9 1071.6 1092.2 1105.6
 Flow (%) = .1 1.8 9.2 18.6 40.8
 Area (sq ft) = 3.8 25.2 81.9 127.3 103.5
 Vel (ft/s) = .26 .61 .96 1.24 3.35
 Depth (ft) = .59 2.17 4.17 6.17 7.70

Station (ft) = 1105.6 1166.5 1176.1
 Flow (%) = 29.3 .1
 Area (sq ft) = 258.3 4.8
 Vel (ft/s) = .97 .23
 Depth (ft) = 4.24 .51

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)
 Total Discharge (cfs, Q) 10125.000
 Computed Water Surface Elevation (ft MSL, CWSEL) 875.50

Station (ft) = 1025.6 1035.8 1051.8 1091.6 1112.5 1116.8
 Flow (%) = .3 3.4 22.5 18.5 10.9
 Area (sq ft) = 7.6 39.8 178.9 123.1 31.1
 Vel (ft/s) = .32 .72 1.07 1.28 2.99
 Depth (ft) = .75 2.49 4.49 5.89 7.29

Station (ft) =	1116.8	1143.7	1167.1	1200.0	1215.4
Flow (%) =	23.9	13.2	7.0		
Area (sq ft) =	158.7	105.1	82.1	11.5	
Vel (ft/s) =	1.28	1.06	.72	.32	
Depth (ft) =	5.89	4.49	2.49	.75	

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)					
Total Discharge (cfs, Q)				10492.000	850
Computed Water Surface Elevation (ft MSL, CWSEL)				876.05	
Station (ft) =	1014.5	1023.5	1038.6	1057.6	1062.3
Flow (%) =	1.2	12.7	29.1	16.1	32.5
Area (sq ft) =	9.2	46.1	82.6	26.3	92.2
Vel (ft/s) =	1.14	2.35	2.99	5.19	2.99
Depth (ft) =	1.02	3.04	4.34	5.64	4.34

Station (ft) =	1083.5	1092.3	1100.6
Flow (%) =	7.3	1.2	
Area (sq ft) =	26.6	8.7	
Vel (ft/s) =	2.32	1.14	
Depth (ft) =	3.04	1.04	

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)					
Total Discharge (cfs, Q)				10912.000	850
Computed Water Surface Elevation (ft MSL, CWSEL)				877.89	
Station (ft) =	1066.8	1095.2	1138.7	1143.1	1181.4
Flow (%) =	5.0	40.9	13.7	36.2	4.2
Area (sq ft) =	26.9	112.5	18.8	99.5	22.8
Vel (ft/s) =	1.58	3.09	6.19	3.09	1.58
Depth (ft) =	.95	2.59	4.29	2.59	.95

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)					
Total Discharge (cfs, Q)				11619.000	850
Computed Water Surface Elevation (ft MSL, CWSEL)				882.67	
Station (ft) =	1050.6	1054.4	1061.9	1088.1	1091.9
Flow (%) =	.1	3.9	51.8	21.2	17.4
Area (sq ft) =	1.3	12.6	96.4	21.6	32.9
Vel (ft/s) =	.92	2.65	4.57	8.35	4.50
Depth (ft) =	.34	1.68	3.68	5.68	3.68

Station (ft) =	1100.9	1110.9	1114.6
Flow (%) =	5.3		.1
Area (sq ft) =	16.8		1.2
Vel (ft/s) =	2.68		.92
Depth (ft) =	1.68		.34

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)
 Total Discharge (cfs, Q) 12068.000
 Computed Water Surface Elevation (ft MSL, CWSEL) 850
 885.31

Station (ft) =	1015.1	1020.1	1031.1	1055.6	1059.6	1082.6
Flow (%) =	.4		6.8		14.6	34.5
Area (sq ft) =	3.3		25.6		22.0	90.4
Vel (ft/s) =	.97		2.26		5.65	3.24
Depth (ft) =	.66		2.32		5.52	3.92

Station (ft) =	1082.6	1093.3	1099.9
Flow (%) =	6.6		.5
Area (sq ft) =	24.8		4.3
Vel (ft/s) =	2.26		.98
Depth (ft) =	2.32		.66

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)
 Total Discharge (cfs, Q) 12460.000
 Computed Water Surface Elevation (ft MSL, CWSEL) 850
 887.40

Station (ft) =	1041.4	1058.2	1082.7	1087.7	1092.2	1097.3
Flow (%) =	2.0		22.4		16.3	8.3
Area (sq ft) =	11.8		58.7		19.7	17.2
Vel (ft/s) =	1.43		3.25		7.02	4.11
Depth (ft) =	.70		2.40		4.40	3.40

Station (ft) =	1097.3	1142.2	1153.6
Flow (%) =	41.4		1.3
Area (sq ft) =	107.9		8.0
Vel (ft/s) =	3.26		1.43
Depth (ft) =	2.40		.70

T1 A-RIVER9601.00
 T2 RIVER FALLS FLOOD INSURANCE STUDY
 T3 MANN VALLEY

JOB PARAMETERS :

J1	ICHECK	INQ	NINV	IDIR	STRT	METRIC	HVINS	Q	WSEL	FQ
0	3	0	0	0.0	0	0.0	0.0	1650.0	864.94	
J2	NPROF	IPLOT	PRFVS	XSECV	XSECH	FN	ALLDC	IBW	CHNIM	ITRACE
2	1	0.0	0.0	0.0	0.0	0.0	-1.0	-6.0	0.0	15

STATUS: Friction loss equation index (IHLEQ) = 1. Therefore friction loss (HL) is calculated as a function of the profile type, which can vary from reach to reach. See documentation for further details.

FLOW DISTRIBUTION :

Cross-Section Number (SECNO) 8638.000
 Total Discharge (cfs, Q) 1650
 Computed Water Surface Elevation (ft MSL, CWSEL) 864.94

Station (ft) = 1012.2 1015.1 1021.0 1034.5 1043.7 1047.6
 Flow (%) = .1 2.7 20.5 24.5 21.5
 Area (sq ft) = 1.3 11.5 53.1 50.9 27.4
 Vel (ft/s) = 1.50 3.85 6.36 7.96 12.97
 Depth (ft) = .47 1.94 3.94 5.49 7.04

Station (ft) = 1047.6 1050.4 1061.8 1073.4 1096.4
 Flow (%) = 7.0 17.2 5.4 1.0
 Area (sq ft) = 15.2 44.8 22.5 10.8
 Vel (ft/s) = 7.61 6.34 3.96 1.55
 Depth (ft) = 5.49 3.94 1.94 .47

FLOW DISTRIBUTION :

Cross-Section Number (SECNO) 8989.000
 Total Discharge (cfs, Q) 1650
 Computed Water Surface Elevation (ft MSL, CWSEL) 873.09

Station (ft) = 1019.7 1022.8 1028.6 1034.2 1037.6 1041.6
 Flow (%) = .2 3.2 9.6 10.8 30.4
 Area (sq ft) = 1.7 12.0 22.9 20.9 32.0
 Vel (ft/s) = 1.81 4.42 6.90 8.50 15.66
 Depth (ft) = .55 2.09 4.09 6.09 8.09

Station (ft) =	1041.6	1048.3	1058.1	1067.9	1073.2
Flow (%) =	22.8	17.1	5.6		.3
Area (sq ft) =	41.2	39.8	20.5	2.9	
Vel (ft/s) =	9.11	7.09	4.53	1.85	
Depth (ft) =	6.09	4.09	2.09		.55

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)
 Total Discharge (cfs, Q) 9256.000
 Computed Water Surface Elevation (ft MSL, CWSEL) 1650
 875.14

Station (ft) =	1023.9	1044.4	1065.5	1086.9	1094.5	1116.8
Flow (%) =	2.0	6.4	12.5	6.4	41.0	
Area (sq ft) =	44.3	87.6	131.1	58.4	199.3	
Vel (ft/s) =	.75	1.21	1.57	1.81	3.39	
Depth (ft) =	2.17	4.14	6.14	7.64	8.97	

Station (ft) =	1116.8	1126.9	1156.0	1176.0
Flow (%) =	8.6	17.0	6.1	
Area (sq ft) =	77.9	178.6	89.3	
Vel (ft/s) =	1.82	1.57	1.13	
Depth (ft) =	7.64	6.14	4.47	

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)
 Total Discharge (cfs, Q) 9536.000
 Computed Water Surface Elevation (ft MSL, CWSEL) 1650
 875.79

Station (ft) =	1263.6	1301.7
Flow (%) =	100.0	
Area (sq ft) =	190.5	
Vel (ft/s) =	8.66	
Depth (ft) =	5.11	

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)
 Total Discharge (cfs, Q) 9576.000
 Computed Water Surface Elevation (ft MSL, CWSEL) 1650
 876.09

Station (ft) =	1338.0	1375.2	1378.0
Flow (%) =	100.0	.0	
Area (sq ft) =	186.3	.3	
Vel (ft/s) =	8.85	.39	
Depth (ft) =	5.09	.10	

Station (ft) =	1062.3	1083.5	1092.3	1100.6	1108.8	1110.5
Flow (%) =	29.3	8.3	3.6			
Area (sq ft) =	142.4	47.2	28.3	11.5		
Vel (ft/s) =	3.40	2.89	2.12	1.17		
Depth (ft) =	6.70	5.40	3.40	1.40		

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)						
Total Discharge (cfs, Q)						10912.000
Computed Water Surface Elevation (ft MSL, CWSEL)						1650
						879.57
Station (ft) =	1051.1	1065.1	1095.2	1138.7	1143.1	1181.4
Flow (%) =	.7	10.9	36.7	9.2	32.4	
Area (sq ft) =	11.1	77.6	185.6	26.2	164.1	
Vel (ft/s) =	1.05	2.32	3.26	5.82	3.26	
Depth (ft) =	.79	2.57	4.27	5.97	4.27	

Station (ft) =	1181.4	1206.9	1223.9
Flow (%) =	9.2	.9	
Area (sq ft) =	65.6	13.4	
Vel (ft/s) =	2.32	1.05	
Depth (ft) =	2.57	.79	

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)						
Total Discharge (cfs, Q)						11619.000
Computed Water Surface Elevation (ft MSL, CWSEL)						1650
						883.73
Station (ft) =	1044.6	1054.4	1061.9	1088.1	1091.9	1100.9
Flow (%) =	1.1	5.6	50.0	17.9	16.8	
Area (sq ft) =	8.5	20.4	124.0	25.6	42.3	
Vel (ft/s) =	2.12	4.52	6.66	11.52	6.56	
Depth (ft) =	.86	2.73	4.73	6.73	4.73	

Station (ft) =	1100.9	1110.9	1120.2
Flow (%) =	7.6	1.0	
Area (sq ft) =	27.3	8.0	
Vel (ft/s) =	4.56	2.12	
Depth (ft) =	2.73	.86	

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)			
Total Discharge (cfs, Q)			12068.000
Computed Water Surface Elevation (ft MSL, CWSEL)			1650
			886.99

Station (ft) =	1006.3	1012.5	1020.1	1031.1	1055.6	1059.6
Flow (%) =	.2	1.9	8.7	34.3	11.7	11.7
Area (sq ft) =	3.1	15.0	44.0	136.8	28.6	28.6
Vel (ft/s) =	.81	2.03	3.27	4.13	6.77	6.77
Depth (ft) =	.49	1.99	3.99	5.59	7.19	7.19
Station (ft) =	1059.6	1082.6	1093.3	1103.3	1108.4	
Flow (%) =	32.3	8.4	2.5	.1		
Area (sq ft) =	128.9	42.7	19.8	2.5		
Vel (ft/s) =	4.13	3.26	2.05	.81		
Depth (ft) =	5.59	3.99	1.99	.49		

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)
 Total Discharge (cfs, Q) 12460.000
 Computed Water Surface Elevation (ft MSL, CWSEL) 1650
 888.82

Station (ft) =	1020.2	1034.3	1058.2	1082.7	1087.7	1092.2
Flow (%) =	.3	6.4	22.3	6.8	11.9	11.9
Area (sq ft) =	5.8	43.7	93.4	24.3	26.0	26.0
Vel (ft/s) =	.89	2.40	3.94	4.61	7.52	7.52
Depth (ft) =	.41	1.82	3.82	4.82	5.82	5.82

Station (ft) =	1092.2	1097.3	1142.2	1158.4	1165.3
Flow (%) =	6.8	41.1	4.3	.2	
Area (sq ft) =	24.4	171.8	29.6	2.8	
Vel (ft/s) =	4.61	3.95	2.40	.89	
Depth (ft) =	4.82	3.82	1.82	.41	

T1 A-RIVER9601.00
 T2 RIVER FALLS FLOOD INSURANCE STUDY
 T3 MANN VALLEY

JOB PARAMETERS :

J1	ICHECK	INQ	NINV	IDIR	STRT	METRIC	HVINS	Q	WSEL	FQ
0	4		0	0	0.0	0	0.0	2000.0	865.43	
J2	NPROF	IPLOT	PRFVS	XSECV	XSECH	FN	ALLDC	IBW	CHNIM	ITRACE
3	1		0.0	0.0	0.0	0.0	-1.0	-6.0	0.0	15

STATUS: Friction loss equation index (IHLEQ) = 1. Therefore friction loss (HL) is calculated as a function of the profile type, which can vary from reach to reach. See documentation for further details.

PROJECT TITLE : RIVER FALLS FLOOD INSURANCE STUDY
 PROJECT NUMBER : A-RIVER9601.00

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Station (ft) =	1116.8	1126.9	1156.0	1176.0
Flow (%) =	8.4	17.2	6.5	
Area (sq ft) =	84.9	198.6	103.1	
Vel (ft/s) =	1.98	1.73	1.26	
Depth (ft) =	8.33	6.83	5.16	

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)
 Total Discharge (cfs, Q) = 9536.000
 Computed Water Surface Elevation (ft MSL, CWSEL) = 876.42

Station (ft) =	1263.5	1301.7	1302.8
Flow (%) =	100.0	.0	
Area (sq ft) =	214.4	.2	
Vel (ft/s) =	9.33	.60	
Depth (ft) =	5.62	.18	

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)
 Total Discharge (cfs, Q) = 9576.000
 Computed Water Surface Elevation (ft MSL, CWSEL) = 876.87

Station (ft) =	1338.0	1338.5	1375.2	1378.0
Flow (%) =	.0	99.9	.1	
Area (sq ft) =	.3	203.0	1.6	
Vel (ft/s) =	.96	9.84	1.20	
Depth (ft) =	.54	5.54	.55	

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)
 Total Discharge (cfs, Q) = 9724.000
 Computed Water Surface Elevation (ft MSL, CWSEL) = 878.86

Station (ft) =	1010.1	1017.6	1029.3	1040.3	1051.9	1071.6
Flow (%) =	.0	.6	1.8	3.8	10.4	
Area (sq ft) =	3.2	21.7	42.6	67.9	154.2	
Vel (ft/s) =	.19	.51	.84	1.11	1.35	
Depth (ft) =	.43	1.86	3.86	5.86	7.86	

Station (ft) =	1071.6	1092.2	1105.6	1166.5	1178.6	1195.0
Flow (%) =	16.0	30.9	32.8	2.1	1.3	
Area (sq ft) =	203.3	153.0	482.9	48.9	41.8	
Vel (ft/s) =	1.57	4.04	1.36	.87	.64	
Depth (ft) =	9.86	11.38	7.93	4.06	2.55	

Station (ft) = 1195.0 1204.3 1210.7
 Flow (%) = .2
 Area (sq ft) = 11.0 2.2
 Vel (ft/s) = .38 .17
 Depth (ft) = 1.18 .34

FLOW DISTRIBUTION :

Cross-Section Number (SECNO) 10125.000
 Total Discharge (cfs, Q) 2000
 Computed Water Surface Elevation (ft MSL, CWSEL) 879.14

Station (ft) = 1004.8 1035.8 1051.8 1091.6 1112.5 1116.8
 Flow (%) = 3.1 5.4 21.7 14.8 7.7
 Area (sq ft) = 83.8 98.0 324.1 199.3 46.7
 Vel (ft/s) = .73 1.10 1.34 1.49 3.30
 Depth (ft) = 2.70 6.14 8.14 9.54 10.94

Station (ft) = 1116.8 1143.7 1167.1 1200.0 1220.6 1237.6
 Flow (%) = 19.1 12.7 11.2 3.6 .7
 Area (sq ft) = 256.9 190.4 202.0 85.4 27.8
 Vel (ft/s) = 1.49 1.33 1.11 .85 .51
 Depth (ft) = 9.54 8.14 6.14 4.14 1.64

FLOW DISTRIBUTION :

Cross-Section Number (SECNO) 10492.000
 Total Discharge (cfs, Q) 2000
 Computed Water Surface Elevation (ft MSL, CWSEL) 879.44

Station (ft) = 1002.0 1007.2 1014.7 1023.5 1038.6 1057.6
 Flow (%) = .1 1.4 4.5 14.7 25.2
 Area (sq ft) = 3.7 18.3 39.1 97.5 147.1
 Vel (ft/s) = .68 1.55 2.32 3.01 3.42
 Depth (ft) = .72 2.44 4.44 6.44 7.74

Station (ft) = 1057.6 1062.3 1083.5 1092.3 1100.6 1108.8
 Flow (%) = 11.6 28.1 8.4 4.3 1.6
 Area (sq ft) = 42.1 164.4 56.3 36.9 20.0
 Vel (ft/s) = 5.52 3.42 2.98 2.32 1.55
 Depth (ft) = 9.04 7.74 6.44 4.44 2.44

Station (ft) = 1108.8 1114.8
 Flow (%) = .1
 Area (sq ft) = 4.3
 Vel (ft/s) = .69
 Depth (ft) = .72

Station (ft) = 1059.6 1082.6 1093.3 1103.3 1111.4
 Flow (%) = 31.5 8.8 3.1 3.1 .4
 Area (sq ft) = 142.1 48.8 25.5 25.5 6.3
 Vel (ft/s) = 4.43 3.59 2.44 1.11 1.11
 Depth (ft) = 6.16 4.56 2.56 2.56 .78

FLOW DISTRIBUTION :

Cross-Section Number (SECNO) 12460.000
 Total Discharge (cfs, Q) 2000
 Computed Water Surface Elevation (ft MSL, CWSEL) 889.34

Station (ft) = 1011.3 1034.3 1058.2 1082.7 1087.7 1092.2
 Flow (%) = 15.9 7.7 21.9 6.4 10.8
 Area (sq ft) = 15.4 56.2 106.1 26.9 28.4
 Vel (ft/s) = 1.19 2.73 4.12 4.74 7.65
 Depth (ft) = .67 2.34 4.34 5.34 6.34

Station (ft) = 1092.2 1097.3 1142.2 1158.4 1169.6
 Flow (%) = 6.4 40.3 5.2 .4
 Area (sq ft) = 27.1 195.2 38.1 7.5
 Vel (ft/s) = 4.74 4.13 2.72 1.18
 Depth (ft) = 5.34 4.34 2.34 .67

T1 A-RIVER9601.00
 T2 RIVER FALLS FLOOD INSURANCE STUDY
 T3 MANN VALLEY

JOB PARAMETERS :

J1	ICHECK	INQ	NINV	IDIR	STRT	METRIC	HVINS	Q	WSEL	FQ
0		5	0	0	0.0	0	0.0	2800.0	866.3	
J2	NPROF	IPLOT	PRPVS	XSECV	XSECH	FN	ALLDC	IBW	CHNIM	ITRACE
15		1	0.0	0.0	0.0	0.0	-1.0	-6.0	0.0	15

STATUS: Friction loss equation index (IHLEQ) = 1. Therefore friction loss (HL) is calculated as a function of the profile type, which can vary from reach to reach. See documentation for further details.

Station (ft) =	1023.9	1044.4	1065.5	1086.9	1094.5	1116.8
Flow (%) =	3.7	8.0	12.9	6.1	36.9	
Area (sq ft) =	85.4	130.2	174.1	73.8	244.0	
Vel (ft/s) =	1.20	1.71	2.07	2.31	4.23	
Depth (ft) =	4.18	6.15	8.15	9.65	10.98	

Station (ft) =	1116.8	1126.9	1156.0	1176.0
Flow (%) =	8.1	17.5	6.9	
Area (sq ft) =	98.4	237.1	129.5	
Vel (ft/s) =	2.31	2.07	1.50	
Depth (ft) =	9.65	8.15	6.48	

FLOW DISTRIBUTION :

Cross-Section Number (SECNO) 9536.000
 Total Discharge (cfs, Q) 2800
 Computed Water Surface Elevation (ft MSL, CWSEL) 877.59

Station (ft) =	1263.5	1301.7	1306.3
Flow (%) =	99.8	.2	
Area (sq ft) =	259.2	3.5	
Vel (ft/s) =	10.78	1.60	
Depth (ft) =	6.79	.77	

FLOW DISTRIBUTION :

Cross-Section Number (SECNO) 9576.000
 Total Discharge (cfs, Q) 2800
 Computed Water Surface Elevation (ft MSL, CWSEL) 877.69

Station (ft) =	1338.0	1338.5	1375.2	1378.0
Flow (%) =	.0	99.6	.4	
Area (sq ft) =	.9	244.9	4.8	
Vel (ft/s) =	1.12	11.39	2.13	
Depth (ft) =	1.69	6.69	1.70	

FLOW DISTRIBUTION :

Cross-Section Number (SECNO) 9724.000
 Total Discharge (cfs, Q) 2800
 Computed Water Surface Elevation (ft MSL, CWSEL) 880.32

Station (ft) =	996.6	1040.3	1051.9	1071.6	1092.2	1105.6
Flow (%) =	3.7	4.1	10.4	15.1	28.3	
Area (sq ft) =	121.1	84.8	182.8	233.4	172.6	
Vel (ft/s) =	.86	1.35	1.59	1.81	4.60	
Depth (ft) =	2.77	7.31	9.31	11.31	12.84	

Station (ft) =	1105.6	1166.5	1195.0	1224.5
Flow (%) =	32.6	4.8		.9
Area (sq ft) =	571.7	132.2	46.1	
Vel (ft/s) =	1.60	1.01	.57	
Depth (ft) =	9.38	4.64	1.57	

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)	10125.000
Total Discharge (cfs, Q)	2800
Computed Water Surface Elevation (ft MSL, CWSEL)	880.63

Station (ft) =	995.9	1035.8	1051.8	1091.6	1112.5	1116.8
Flow (%) =	4.4	5.7	21.1	13.9	7.0	
Area (sq ft) =	136.4	121.7	383.4	230.4	53.1	
Vel (ft/s) =	.90	1.31	1.54	1.69	3.70	
Depth (ft) =	3.42	7.63	9.63	11.03	12.43	

Station (ft) =	1116.8	1143.7	1167.1	1200.0	1220.6	1244.6
Flow (%) =	17.9	12.4	11.8	4.5	1.4	
Area (sq ft) =	297.0	225.2	251.0	116.1	58.2	
Vel (ft/s) =	1.69	1.54	1.32	1.07	.69	
Depth (ft) =	11.03	9.63	7.63	5.63	2.43	

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)	10492.000
Total Discharge (cfs, Q)	2800
Computed Water Surface Elevation (ft MSL, CWSEL)	880.91

Station (ft) =	996.1	1023.5	1038.6	1057.6	1062.3	1083.5
Flow (%) =	8.0	14.6	23.8	10.6	26.6	
Area (sq ft) =	96.9	119.9	175.1	49.0	195.7	
Vel (ft/s) =	2.32	3.42	3.80	6.05	3.80	
Depth (ft) =	3.54	7.91	9.21	10.51	9.21	

Station (ft) =	1083.5	1092.3	1100.6	1117.1	1121.4
Flow (%) =	8.3	4.9	3.2	.0	
Area (sq ft) =	69.2	49.2	48.0	2.0	
Vel (ft/s) =	3.38	2.78	1.84	.51	
Depth (ft) =	7.91	5.91	2.90	.46	

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)	10912.000
Total Discharge (cfs, Q)	2800
Computed Water Surface Elevation (ft MSL, CWSEL)	881.75

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Station (ft) =	1038.3	1047.3	1065.1	1095.2	1138.7	1143.1
Flow (%) =	.2	3.2	13.4	32.2	32.2	6.9
Area (sq ft) =	7.8	49.0	142.9	279.8	279.8	35.7
Vel (ft/s) =	.84	1.82	2.63	3.23	3.23	5.39
Depth (ft) =	.87	2.74	4.74	6.44	6.44	8.14
Station (ft) =	1143.1	1181.4	1206.9	1228.6	1243.2	
Flow (%) =	28.5	11.3	3.9	.4		
Area (sq ft) =	247.4	120.9	59.2	12.8		
Vel (ft/s) =	3.23	2.63	1.82	.85		
Depth (ft) =	6.44	4.74	2.74	.87		

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)
 Total Discharge (cfs, Q) 11619.000
 Computed Water Surface Elevation (ft MSL, CWSEL) 884.87

Station (ft) =	1038.5	1043.1	1054.4	1061.9	1088.1	1091.9
Flow (%) =	.1	3.0	6.6	47.2	47.2	15.3
Area (sq ft) =	2.0	21.2	29.0	154.0	154.0	30.0
Vel (ft/s) =	1.50	3.97	6.37	8.58	8.58	14.27
Depth (ft) =	.44	1.87	3.87	5.87	5.87	7.87

Station (ft) =	1091.9	1100.9	1110.9	1121.7	1127.0	
Flow (%) =	15.9	8.9	2.9	.1		
Area (sq ft) =	52.6	38.8	20.2	2.3		
Vel (ft/s) =	8.46	6.43	3.97	1.51		
Depth (ft) =	5.87	3.87	1.87	.44		

FLOW DISTRIBUTION :

Cross-Section Number (SECNO)
 Total Discharge (cfs, Q) 12068.000
 Computed Water Surface Elevation (ft MSL, CWSEL) 888.67

Station (ft) =	989.3	1000.0	1012.5	1020.1	1031.1	1055.6
Flow (%) =	.1	1.4	3.1	9.4	9.4	31.8
Area (sq ft) =	3.6	20.9	27.7	62.5	62.5	177.9
Vel (ft/s) =	.64	1.86	3.10	4.20	4.20	5.01
Depth (ft) =	.33	1.67	3.67	5.67	5.67	7.27

Station (ft) =	1055.6	1059.6	1082.6	1093.3	1103.3	1113.7
Flow (%) =	10.0	30.0	9.1	4.1	4.1	1.1
Area (sq ft) =	35.3	167.5	60.6	36.5	36.5	17.3
Vel (ft/s) =	7.93	5.01	4.20	3.13	3.13	1.85
Depth (ft) =	8.87	7.27	5.67	3.67	3.67	1.67

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Station (ft) = 1113.7 1117.3
 Flow (%) = .0
 Area (sq ft) = 1.2
 Vel (ft/s) = .63
 Depth (ft) = .33

FLOW DISTRIBUTION :

Cross-Section Number (SECNO) 12460.000
 Total Discharge (cfs, Q) 2800
 Computed Water Surface Elevation (ft MSL, CWSEL) 890.38

Station (ft) = 989.1 1000.0 1034.3 1058.2 1082.7 1087.7
 Flow (%) = .0 3.0 9.4 20.8 5.7
 Area (sq ft) = 2.1 47.4 81.1 131.6 32.2
 Vel (ft/s) = .48 1.79 3.24 4.42 4.96
 Depth (ft) = .19 1.38 3.38 5.38 6.38

Station (ft) = 1087.7 1092.2 1097.3 1142.2 1158.4 1175.0
 Flow (%) = 9.3 5.7 38.2 6.3 1.5
 Area (sq ft) = 33.0 32.4 242.0 55.0 23.0
 Vel (ft/s) = 7.86 4.96 4.43 3.23 1.78
 Depth (ft) = 7.38 6.38 5.38 3.38 1.38

Station (ft) = 1175.0 1180.7
 Flow (%) = .0
 Area (sq ft) = 1.1
 Vel (ft/s) = .48
 Depth (ft) = .19

SPECIAL NOTE :

An asterisk (*) to the left of the cross-section number indicates a special note is present in the SUMMARY OF WARNING AND STATUS MESSAGES section.

SUMMARY PRINTOUT : RIVER FALLS FLOOD INSURANCE STUDY
 MANN VALLEY
 A-RIVER9601.00

Cross-Section Number	Discharge Flow (cfs)	Computed W. S. Elevation (ft MSL)	W. S. Diff per Section (ft)	Energy Gradient Elevation (ft MSL)	Energy Elev pre Prof. (ft)	Water Surface Top Width (ft)	Up/Down Conyance Ratio	Critical W. S. Elevation (ft MSL)
SECNO	Q	CWSEL	DIFWSX	EG	DIFEG	TOPWID	KRATIO	CRISWS
8638.000	850.00	863.38	.00	864.27	.00	52.88	.00	863.24
8638.000	1650.00	864.94	.00	865.98	1.71	84.22	.00	864.49
8638.000	2000.00	865.43	.00	866.52	2.25	97.72	.00	865.03
8638.000	2800.00	866.30	.00	867.50	3.22	116.79	.00	865.93
* 8989.000	850.00	871.44	8.06	872.79	.00	40.66	.98	871.44
* 8989.000	1650.00	873.09	8.15	874.80	2.00	53.48	.91	873.09
* 8989.000	2000.00	873.63	8.20	875.47	2.68	57.66	.90	873.63
* 8989.000	2800.00	874.71	8.41	876.77	3.98	66.10	.90	874.71
* 9256.000	850.00	873.09	1.66	873.16	.00	143.19	6.23	869.50
* 9256.000	1650.00	875.14	2.04	875.23	2.07	152.08	5.98	870.82
* 9256.000	2000.00	875.83	2.20	875.93	2.78	152.08	5.89	871.13
* 9256.000	2800.00	877.15	2.44	877.29	4.13	152.08	5.58	871.85
* 9536.000	850.00	873.80	.70	874.54	.00	30.60	.24	872.37
* 9536.000	1650.00	875.79	.66	876.96	2.41	37.30	.24	874.30
* 9536.000	2000.00	876.42	.60	877.77	3.23	39.26	.25	874.99
* 9536.000	2800.00	877.59	.44	879.39	4.85	42.78	.26	876.25
9576.000	850.00	874.06	.26	874.86	.00	29.80	.99	872.74
9576.000	1650.00	876.09	.30	877.31	2.45	39.99	1.00	874.70
9576.000	2000.00	876.87	.44	878.37	3.51	39.99	.64	875.37
9576.000	2800.00	877.69	.10	879.70	4.84	39.99	.96	875.00
* 9724.000	850.00	875.17	1.11	875.25	.00	142.18	3.45	871.25
* 9724.000	1650.00	877.72	1.62	877.82	2.56	180.80	3.68	872.55
* 9724.000	2000.00	878.86	1.99	878.95	3.70	200.69	6.16	873.05
* 9724.000	2800.00	880.32	2.62	880.43	5.18	227.86	3.98	873.97
10125.000	850.00	875.50	.33	875.53	.00	189.78	.94	871.96

PROJECT TITLE : RIVER FALLS FLOOD INSURANCE STUDY

PROJECT NUMBER : A-RIVER9601.00

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Cross- Section Number	Discharge Flow (cfs)	Computed W. S. Elevation (ft MSL)	W. S. Elev Diff per Section (ft)	Energy Gradient Elevation (ft MSL)	Energy Elev Diff pre Prof. (ft)	Water Surface Top Width (ft)	Up/Down Conveyance Ratio	Critical W. S. Elevation (ft MSL)
SECNO	Q	CWSEL	DIFWSX	EG	DIFEG	TOPWID	KRATIO	CRIWS
10125.000	1650.00	878.03	.31	878.07	2.54	221.38	1.03	872.85
10125.000	2000.00	879.14	.28	879.17	3.64	232.78	1.04	873.15
10125.000	2800.00	880.63	.31	880.67	5.14	248.64	1.06	873.60
* 10492.000	850.00	876.05	.55	876.21	.00	86.26	.50	874.02
* 10492.000	1650.00	878.40	.37	878.59	2.38	104.74	.53	874.99
* 10492.000	2000.00	879.44	.30	879.62	3.41	112.76	.54	875.38
* 10492.000	2800.00	880.91	.28	881.12	4.91	125.31	.56	876.14
* 10912.000	850.00	877.89	1.85	878.08	.00	138.79	.69	876.99
10912.000	1650.00	879.57	1.17	879.74	1.66	172.89	.77	877.71
10912.000	2000.00	880.37	.94	880.52	2.44	186.36	.82	877.95
10912.000	2800.00	881.75	.83	881.89	3.81	204.89	.93	878.50
11619.000	850.00	882.67	4.78	883.11	.00	64.00	.85	881.61
* 11619.000	1650.00	883.73	4.16	884.54	1.43	75.61	.52	882.85
* 11619.000	2000.00	884.06	3.68	885.03	1.92	79.27	.42	883.30
* 11619.000	2800.00	884.87	3.13	886.09	2.97	88.44	.35	884.18
* 12068.000	850.00	885.31	2.64	885.50	.00	84.80	1.47	883.54
* 12068.000	1650.00	886.99	3.26	887.26	1.76	102.09	1.80	884.56
* 12068.000	2000.00	887.56	3.51	887.86	2.37	108.66	1.92	884.92
* 12068.000	2800.00	888.67	3.80	889.04	3.54	127.99	1.98	885.64
12460.000	850.00	887.40	2.09	887.66	.00	112.14	.72	886.63
12460.000	1650.00	888.82	1.83	889.11	1.45	145.09	.81	887.47
12460.000	2000.00	889.34	1.77	889.63	1.97	158.33	.85	887.64
12460.000	2800.00	890.38	1.71	890.69	3.03	191.60	.93	888.29

SUMMARY PRINTOUT : RIVER FALLS FLOOD INSURANCE STUDY

 MANN VALLEY
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Cross- Section Number	Discharge Flow (cfs)	Computed W. S. Elevation (ft MSL) CWSEL	W. S. Elev Diff per Know/Comp (ft) DIFKWS	Flow Depth (ft) DEPTH	Channel Reach Length (ft) XLCH	Channel Flow (cfs) QCH	Left Overbank Flow (cfs) QLOB	Right Overbank Flow (cfs) QROB
8638.000	850.00	863.38	.00	6.48	.00	243.45	398.47	208.08
8638.000	1650.00	864.94	.00	8.04	.00	355.31	788.59	506.10
8638.000	2000.00	865.43	.00	8.53	.00	396.36	945.03	658.61
8638.000	2800.00	866.30	.00	9.40	.00	478.45	1273.27	1048.28
* 8989.000	850.00	871.44	.00	7.44	351.46	333.94	173.27	342.78
* 8989.000	1650.00	873.09	.00	9.09	351.46	501.96	391.83	756.21
* 8989.000	2000.00	873.63	.00	9.63	351.46	565.63	491.54	942.83
* 8989.000	2800.00	874.71	.00	10.71	351.46	691.64	727.52	1380.84
* 9256.000	850.00	875.14	.00	9.09	266.29	410.45	190.67	248.88
* 9256.000	1650.00	875.14	.00	11.14	266.29	675.68	450.84	523.48
* 9256.000	2000.00	875.83	.00	11.83	266.29	785.68	572.14	642.18
* 9256.000	2800.00	877.15	.00	13.15	266.29	1031.81	855.71	912.48
* 9536.000	850.00	873.80	.00	7.80	280.72	850.00	.00	.00
* 9536.000	1650.00	875.79	.00	9.79	280.72	1650.00	.00	.00
* 9536.000	2000.00	876.42	.00	10.42	280.72	1999.88	.00	.12
* 9536.000	2800.00	877.59	.00	11.59	280.72	2794.32	.00	5.68
* 9576.000	850.00	874.06	.00	8.06	40.00	850.00	.00	.00
* 9576.000	1650.00	876.09	.00	10.09	40.00	1649.89	.00	.11
* 9576.000	2000.00	876.87	.00	10.87	40.00	1997.85	.28	1.87
* 9576.000	2800.00	877.69	.00	11.69	40.00	2788.78	1.00	10.22
* 9724.000	850.00	875.17	.00	9.47	148.29	346.88	252.55	250.56
* 9724.000	1650.00	877.72	.00	12.02	148.29	549.68	526.01	574.31
* 9724.000	2000.00	878.86	.00	13.16	148.29	618.23	651.07	730.70
* 9724.000	2800.00	880.32	.00	14.62	148.29	793.43	932.42	1074.15
10125.000	850.00	875.50	.00	8.30	400.84	93.05	379.26	377.69
10125.000	1650.00	878.03	.00	10.83	400.84	138.60	741.09	770.31
10125.000	2000.00	879.14	.00	11.94	400.84	154.26	899.20	946.54
10125.000	2800.00	880.63	.00	13.43	400.84	196.44	1261.24	1342.32
* 10492.000	850.00	876.05	.00	6.65	366.80	136.48	365.81	347.72
* 10492.000	1650.00	878.40	.00	9.00	366.80	207.62	748.42	693.95

Cross-Section Number	Discharge Flow (cfs) Q	Computed W. S. Elevation (ft MSL) CWSEL	W. S. Diff per Know/Comp (ft) DIFKWS	Elev per Depth (ft) DEPTH	Channel Reach Length (ft) XLCH	Channel Flow (cfs) QCH	Left Overbank Flow (cfs) QLOB	Right Overbank Flow (cfs) QROB
* 10492.000	2000.00	879.44	.00	10.04	366.80	232.64	918.17	849.19
* 10492.000	2800.00	880.91	.00	11.51	366.80	296.14	1300.39	1203.48
* 10912.000	850.00	877.89	.00	5.29	420.48	116.27	390.30	343.44
10912.000	1650.00	879.57	.00	6.97	420.48	152.38	796.52	701.11
10912.000	2000.00	880.37	.00	7.77	420.48	162.71	974.46	862.83
10912.000	2800.00	881.75	.00	9.15	420.48	192.26	1373.59	1234.15
* 11619.000	850.00	882.67	.00	6.67	706.87	180.53	475.11	194.35
* 11619.000	1650.00	883.73	.00	7.73	706.87	295.18	935.49	419.33
* 11619.000	2000.00	884.06	.00	8.06	706.87	341.40	1136.36	522.24
* 11619.000	2800.00	884.87	.00	8.87	706.87	428.07	1594.13	777.80
* 12068.000	850.00	885.31	.00	6.51	448.72	124.15	372.33	353.52
* 12068.000	1650.00	886.99	.00	8.19	448.72	193.79	741.90	714.31
* 12068.000	2000.00	887.56	.00	8.76	448.72	221.38	904.52	874.11
* 12068.000	2800.00	888.67	.00	9.87	448.72	279.68	1280.02	1240.31
12460.000	850.00	887.40	.00	5.40	392.45	138.15	278.10	433.75
12460.000	1650.00	888.82	.00	6.82	392.45	195.69	590.23	864.08
12460.000	2000.00	889.34	.00	7.34	392.45	216.80	736.70	1046.50
12460.000	2800.00	890.38	.00	8.38	392.45	259.54	1089.76	1450.71

SUMMARY PRINTOUT : RIVER FALLS FLOOD INSURANCE STUDY
 MANN VALLEY
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Cross-Section Number	Computed W. S. Elevation (ft MSL) CWSEL	Left Overbank Flow (cfs) QLOB	Left Overbank Velocity (ft/s) VLOB	Left Manning n * 1,000 K*XL	Channel Flow (cfs) QCH	Channel Mean Velocity (ft/s) VCH	Channel Manning n * 1,000 K*XNCH	Right Overbank Flow (cfs) QROB	Right Overbank Velocity (ft/s) VROB	Right Manning n * 1,000 K*XNR
8638.000	863.38	398.47	5.59	70.00	243.45	11.42	40.00	208.08	4.78	70.00
8638.000	864.94	788.59	6.75	70.00	355.31	12.97	40.00	506.10	5.42	70.00
8638.000	865.43	945.03	7.13	70.00	396.36	13.53	40.00	658.61	5.47	70.00
8638.000	866.30	1273.27	7.84	70.00	478.45	14.64	40.00	1048.28	5.77	70.00
* 8989.000	871.44	173.27	5.44	70.00	333.94	13.10	40.00	342.78	5.83	70.00
* 8989.000	873.09	391.83	6.81	70.00	501.96	15.66	40.00	756.21	7.24	70.00

Cross- Section Number	Computed W. S. Elevation (ft MSL) CWSEL	Left Overbank Flow (cfs) QLOB	Left Overbank Velocity (ft/s) VLOB	Left Overbank Manning n * 1,000 K*XL	Channel Flow (cfs) QCH	Channel Mean Flow Velocity (ft/s) VCH	Channel Manning n * 1,000 K*XC	Right Overbank Flow (cfs) QROB	Right Overbank Velocity (ft/s) VROB	Right Overbank Manning n * 1,000 K*XNR
* 8989.000	873.63	491.54	7.27	70.00	565.63	16.55	40.00	942.83	7.71	70.00
* 8989.000	874.71	727.52	8.06	70.00	691.64	17.98	40.00	1380.84	8.52	70.00
* 9256.000	873.09	190.67	1.05	70.00	410.45	2.67	40.00	248.88	1.11	70.00
* 9256.000	875.14	450.84	1.40	70.00	675.68	3.39	40.00	523.48	1.51	70.00
* 9256.000	875.83	572.14	1.55	70.00	785.68	3.66	40.00	642.18	1.66	70.00
* 9256.000	877.15	855.71	1.85	70.00	1031.81	4.23	40.00	912.48	1.96	70.00
* 9536.000	873.80	.00	.00	.00	850.00	6.92	40.00	.00	.00	.00
* 9536.000	875.79	.00	.00	.00	1650.00	8.66	40.00	.00	.00	.00
* 9536.000	876.42	.00	.00	.00	1999.88	9.33	40.00	.12	.60	70.00
* 9536.000	877.59	.00	.00	.00	2794.32	10.78	40.00	5.68	1.60	70.00
* 9576.000	874.06	.00	.00	.00	850.00	7.17	40.00	.00	.00	.00
* 9576.000	876.09	.00	.02	70.00	1649.89	8.85	40.00	.11	.39	70.00
* 9576.000	876.87	.28	.96	70.00	1997.85	9.84	40.00	1.87	1.20	70.00
* 9576.000	877.69	1.00	1.12	70.00	2788.78	11.39	40.00	10.22	2.13	70.00
* 9724.000	875.17	252.55	1.06	100.00	346.88	3.35	40.00	250.56	.95	100.00
* 9724.000	877.72	526.01	1.30	100.00	549.68	3.99	40.00	574.31	1.21	100.00
* 9724.000	878.86	651.07	1.32	100.00	618.23	4.04	40.00	730.70	1.25	100.00
* 9724.000	880.32	932.42	1.50	100.00	793.43	4.60	40.00	1074.15	1.43	100.00
10125.000	875.50	379.26	1.09	100.00	93.05	2.99	40.00	377.69	1.06	100.00
10125.000	878.03	741.09	1.26	100.00	138.60	3.30	40.00	770.31	1.22	100.00
10125.000	879.14	899.20	1.28	100.00	154.26	3.30	40.00	946.54	1.24	100.00
10125.000	880.63	1261.24	1.45	100.00	196.44	3.70	40.00	1342.32	1.42	100.00
* 10492.000	876.05	365.81	2.65	70.00	136.48	5.19	40.00	347.72	2.73	70.00
* 10492.000	878.40	748.42	2.99	70.00	207.62	5.57	40.00	693.95	3.02	70.00
* 10492.000	879.44	918.17	3.00	70.00	232.64	5.52	40.00	849.19	3.01	70.00
* 10492.000	880.91	1300.39	3.32	70.00	296.14	6.05	40.00	1203.48	3.31	70.00
* 10912.000	877.89	390.30	2.80	70.00	116.27	6.19	40.00	343.44	2.81	70.00
10912.000	879.57	796.52	2.90	70.00	152.38	5.82	40.00	701.11	2.88	70.00
10912.000	880.37	974.46	2.81	70.00	162.71	5.48	40.00	862.83	2.77	70.00
10912.000	881.75	1373.59	2.87	70.00	192.26	5.39	40.00	1234.15	2.80	70.00
11619.000	882.67	475.11	4.31	70.00	180.53	8.35	40.00	194.35	3.81	70.00
* 11619.000	883.73	935.49	6.12	70.00	295.18	11.52	40.00	419.33	5.40	70.00
* 11619.000	884.06	1136.36	6.78	70.00	341.40	12.70	40.00	522.24	5.98	70.00
* 11619.000	884.87	1594.13	7.73	70.00	428.07	14.27	40.00	777.80	6.83	70.00
* 12068.000	885.31	372.33	2.98	70.00	124.15	5.65	40.00	353.52	2.96	70.00

PROJECT TITLE : RIVER FALLS FLOOD INSURANCE STUDY

11/12/1997

PROJECT NUMBER : A-RIVER9601.00

Cross-Section Number	Computed W. S. Elevation (ft MSL) CWSEL	Left Overbank Flow (cfs) QLOB	Left Overbank Velocity (ft/s) VLOB	Left Manning n * 1,000 K*XLN	Channel Flow (cfs) QCH	Channel Mean Velocity (ft/s) VCH	Channel Manning n * 1,000 K*XNCH	Right Overbank Flow (cfs) QROB	Right Overbank Velocity (ft/s) VROB	Right Manning n * 1,000 K*XNR
* 12068.000	886.99	741.90	3.73	70.00	193.79	6.77	40.00	714.31	3.68	70.00
* 12068.000	887.56	904.52	3.96	70.00	221.38	7.16	40.00	874.11	3.92	70.00
* 12068.000	888.67	1280.02	4.38	70.00	279.68	7.93	40.00	1240.31	4.38	70.00
12460.000	887.40	278.10	3.18	70.00	138.15	7.02	40.00	433.75	3.26	70.00
12460.000	888.82	590.23	3.53	70.00	195.69	7.52	40.00	864.08	3.78	70.00
12460.000	889.34	736.70	3.60	70.00	216.80	7.65	40.00	1046.50	3.91	70.00
12460.000	890.38	1089.76	3.70	70.00	259.54	7.86	40.00	1450.71	4.11	70.00

SUMMARY PRINTOUT TABLE 150 : RIVER FALLS FLOOD INSURANCE STUDY

MANN VALLEY
A-RIVER9601.00

Cross-Section Number	Channel Reach Length (ft) XLCH	Top of Roadway Elevation (ft MSL) ELTRD	Max. Chord Elevation (ft MSL) ELLC	Minimum Elevation (ft MSL) ELMIN	Discharge Flow (cfs) Q	Computed W. S. Elevation (ft MSL) CWSEL	Critical W. S. Elevation (ft MSL) CRISWS	Energy Gradient Elevation (ft MSL) EG	Energy Gradient Slope * 10,000 10K*S	Channel Mean Velocity (ft/s) VCH	Cross-Section Area (sq ft) AREA	Index (0.01) Convey
* 8638.000	.00	.00	.00	856.90	850.00	863.38	863.24	864.27	158.35	11.42	136.12	67
* 8638.000	.00	.00	.00	856.90	1650.00	864.94	864.49	865.98	146.34	12.97	237.58	136
* 8638.000	.00	.00	.00	856.90	2000.00	865.43	865.03	866.52	145.53	13.53	282.15	165
* 8638.000	.00	.00	.00	856.90	2800.00	866.30	865.93	867.50	147.28	14.64	376.86	230
* 8989.000	351.46	.00	.00	864.00	850.00	871.44	871.44	872.79	165.97	13.10	116.18	65
* 8989.000	351.46	.00	.00	864.00	1650.00	873.09	873.09	874.80	175.03	15.66	194.02	124
* 8989.000	351.46	.00	.00	864.00	2000.00	873.63	873.63	875.47	179.17	16.55	224.04	145
* 8989.000	351.46	.00	.00	864.00	2800.00	874.71	874.71	876.77	180.83	17.98	290.87	205
* 9256.000	266.29	.00	.00	864.00	850.00	873.09	869.50	873.16	4.27	2.67	559.96	411
* 9256.000	266.29	.00	.00	864.00	1650.00	875.14	870.82	875.23	4.90	3.39	866.42	745
* 9256.000	266.29	.00	.00	864.00	2000.00	875.83	871.13	875.93	5.17	3.66	971.42	875
* 9256.000	266.29	.00	.00	864.00	2800.00	877.15	871.85	877.29	5.82	4.23	1172.22	1161
* 9536.000	280.72	.00	.00	866.00	850.00	873.80	872.37	874.54	71.76	6.92	122.76	100
* 9536.000	280.72	.00	.00	866.00	1650.00	875.79	874.30	876.96	81.81	8.66	190.51	182
* 9536.000	280.72	.00	.00	866.00	2000.00	876.42	874.99	877.77	83.52	9.33	214.63	218
* 9536.000	280.72	.00	.00	866.00	2800.00	877.59	876.25	879.39	86.64	10.78	262.77	300
9576.000	40.00	880.30	876.55	866.00	850.00	874.06	872.74	874.86	73.91	7.17	118.54	95

Cross- Section Number	Channel Reach Length (ft) XLCH	Top of Roadway Elevation (ft MSL) ELTRD	Max. Low Chord Elevation (ft MSL) ELLC	Minimum C. S. Elevation (ft MSL) ELMIN	Discharge Flow (cfs) Q	Computed W. S. Elevation (ft MSL) CWSEL	Critical W. S. Elevation (ft MSL) CRIWS	Energy Gradient Elevation (ft MSL) EG	Energy Gradient Slope * 10,000 10K*S	Channel Mean Flow Velocity (ft/s) VCH	Cross- Section Area (sq ft) AREA	Index (0.01 Convey .01K
* 9576.000	40.00	880.30	876.55	866.00	1650.00	876.09	874.70	877.31	81.17	8.85	186.65	183
* 9576.000	40.00	880.30	876.55	866.00	2000.00	876.87	875.37	878.37	202.68	9.84	204.80	140
* 9576.000	40.00	880.30	876.55	866.00	2800.00	877.69	.00	879.70	93.23	11.39	250.59	283
* 9724.000	148.29	.00	.00	865.70	850.00	875.17	871.25	875.25	6.20	3.35	604.68	341
* 9724.000	148.29	.00	.00	865.70	1650.00	877.72	872.55	877.82	6.00	3.99	1015.54	673
* 9724.000	148.29	.00	.00	865.70	2000.00	878.86	873.05	878.95	5.34	4.04	1232.66	865
* 9724.000	148.29	.00	.00	865.70	2800.00	880.32	873.97	880.43	5.89	4.60	1544.76	1153
10125.000	400.84	.00	.00	867.20	850.00	875.50	871.96	875.53	6.96	2.99	738.07	322
10125.000	400.84	.00	.00	867.20	1650.00	878.03	872.85	878.07	5.71	3.30	1261.93	690
10125.000	400.84	.00	.00	867.20	2000.00	879.14	873.15	879.17	4.95	3.30	1514.23	890
10125.000	400.84	.00	.00	867.20	2800.00	880.63	873.60	880.67	5.25	3.70	1872.47	1222
* 10492.000	366.80	.00	.00	869.40	850.00	876.05	874.02	876.21	28.07	5.19	291.66	160
* 10492.000	366.80	.00	.00	869.40	1650.00	878.40	874.99	878.59	20.27	5.57	517.03	366
* 10492.000	366.80	.00	.00	869.40	2000.00	879.44	875.38	879.62	16.96	5.52	629.71	485
* 10492.000	366.80	.00	.00	869.40	2800.00	880.91	876.14	881.12	16.61	6.05	804.81	686
* 10912.000	420.48	.00	.00	872.60	850.00	877.89	876.99	878.08	59.66	6.19	280.48	110
10912.000	420.48	.00	.00	872.60	1650.00	879.57	877.71	879.74	34.00	5.82	543.51	282
10912.000	420.48	.00	.00	872.60	2000.00	880.37	877.95	880.52	25.50	5.48	687.66	396
10912.000	420.48	.00	.00	872.60	2800.00	881.75	878.50	881.89	19.28	5.39	955.34	637
11619.000	706.87	.00	.00	876.00	850.00	882.67	881.61	883.11	81.93	8.35	182.90	95
* 11619.000	706.87	.00	.00	876.00	1650.00	883.73	882.85	884.54	124.41	11.52	256.18	147
* 11619.000	706.87	.00	.00	876.00	2000.00	884.06	883.30	885.03	141.90	12.70	281.68	167
* 11619.000	706.87	.00	.00	876.00	2800.00	884.87	884.18	886.09	154.92	14.27	350.09	224
* 12068.000	448.72	.00	.00	878.80	850.00	885.31	883.54	885.50	37.89	5.65	266.21	138
* 12068.000	448.72	.00	.00	878.80	1650.00	886.99	884.56	887.26	38.21	6.77	421.53	266
* 12068.000	448.72	.00	.00	878.80	2000.00	887.56	884.92	887.86	38.60	7.16	481.97	321
* 12068.000	448.72	.00	.00	878.80	2800.00	888.67	885.64	889.04	39.58	7.93	610.91	445
12460.000	392.45	.00	.00	882.00	850.00	887.40	886.63	887.66	73.35	7.02	240.35	90
12460.000	392.45	.00	.00	882.00	1650.00	888.82	887.47	889.11	57.90	7.52	421.83	216
12460.000	392.45	.00	.00	882.00	2000.00	889.34	887.64	889.63	53.41	7.65	500.85	273
12460.000	392.45	.00	.00	882.00	2800.00	890.38	888.29	890.69	46.12	7.86	680.71	412

SUMMARY PRINTOUT TABLE 150 : RIVER FALLS FLOOD INSURANCE STUDY
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Cross-Section Number	Discharge Flow (cfs)	Computed W.S. Elevation (ft MSL)	W.S. Elev Profile (ft)	W.S. Elev Section (ft)	W.S. Elev per Know/Comp (ft)	Elev per Diff/Comp (ft)	Water Surface Top Width (ft)	Channel Reach Length (ft)
SECNO	Q	CWSEL	DIFWSP	DIFWSX	DIFKWS	TOPWID	XLCH	
* 8638.000	850.00	863.38	.00	.00	.00	.00	52.88	.00
* 8638.000	1650.00	864.94	1.56	.00	.00	.00	84.22	.00
* 8638.000	2000.00	865.43	.49	.00	.00	.00	97.72	.00
* 8638.000	2800.00	866.30	.87	.00	.00	.00	116.79	.00
* 8989.000	850.00	871.44	.00	8.06	.00	.00	40.66	351.46
* 8989.000	1650.00	873.09	1.65	8.15	.00	.00	53.48	351.46
* 8989.000	2000.00	873.63	.54	8.20	.00	.00	57.66	351.46
* 8989.000	2800.00	874.71	1.08	8.41	.00	.00	66.10	351.46
* 9256.000	850.00	873.09	.00	1.66	.00	.00	143.19	266.29
* 9256.000	1650.00	875.14	2.04	2.04	.00	.00	152.08	266.29
* 9256.000	2000.00	875.83	.69	2.20	.00	.00	152.08	266.29
* 9256.000	2800.00	877.15	1.32	2.44	.00	.00	152.08	266.29
* 9536.000	850.00	873.80	.00	.70	.00	.00	30.60	280.72
* 9536.000	1650.00	875.79	1.99	.66	.00	.00	37.30	280.72
* 9536.000	2000.00	876.42	.63	.60	.00	.00	39.26	280.72
* 9536.000	2800.00	877.59	1.16	.44	.00	.00	42.78	280.72
* 9576.000	850.00	874.06	.00	.26	.00	.00	29.80	40.00
* 9576.000	1650.00	876.09	2.03	.30	.00	.00	39.99	40.00
* 9576.000	2000.00	876.87	.78	.44	.00	.00	39.99	40.00
* 9576.000	2800.00	877.69	.82	.10	.00	.00	39.99	40.00
* 9724.000	850.00	875.17	.00	1.11	.00	.00	142.18	148.29
* 9724.000	1650.00	877.72	2.55	1.62	.00	.00	180.80	148.29
* 9724.000	2000.00	878.86	1.14	1.99	.00	.00	200.69	148.29
* 9724.000	2800.00	880.32	1.46	2.62	.00	.00	227.86	148.29
10125.000	850.00	875.50	.00	.33	.00	.00	189.78	400.84
10125.000	1650.00	878.03	2.53	.31	.00	.00	221.38	400.84
10125.000	2000.00	879.14	1.11	.28	.00	.00	232.78	400.84
10125.000	2800.00	880.63	1.49	.31	.00	.00	248.64	400.84
* 10492.000	850.00	876.05	.00	.55	.00	.00	86.26	366.80
* 10492.000	1650.00	878.40	2.35	.37	.00	.00	104.74	366.80

Cross-Section Number	Discharge Flow (cfs)	Computed W. S. Elevation (ft MSL)	W.S. Elev Diff per Profile (ft)	W.S. Elev Diff per Section (ft)	W.S. Elev Diff per Know/Comp (ft)	Water Surface Top Width (ft)	Channel Reach Length (ft)
SECNO	Q	DIFWSP	DIFWSX	DIFKWS	TOPWID	XLCH	
* 10492.000	2000.00	879.44	1.04	.30	.00	112.76	366.80
* 10492.000	2800.00	880.91	1.47	.28	.00	125.31	366.80
* 10912.000	850.00	877.89	.00	1.85	.00	138.79	420.48
10912.000	1650.00	879.57	1.68	1.17	.00	172.89	420.48
10912.000	2000.00	880.37	.80	.94	.00	186.36	420.48
10912.000	2800.00	881.75	1.37	.83	.00	204.89	420.48
11619.000	850.00	882.67	.00	4.78	.00	64.00	706.87
* 11619.000	1650.00	883.73	1.06	4.16	.00	75.61	706.87
* 11619.000	2000.00	884.06	.33	3.68	.00	79.27	706.87
* 11619.000	2800.00	884.87	.82	3.13	.00	88.44	706.87
* 12068.000	850.00	885.31	.00	2.64	.00	84.80	448.72
* 12068.000	1650.00	886.99	1.68	3.26	.00	102.09	448.72
* 12068.000	2000.00	887.56	.57	3.51	.00	108.66	448.72
* 12068.000	2800.00	888.67	1.11	3.80	.00	127.99	448.72
12460.000	850.00	887.40	.00	2.09	.00	112.14	392.45
12460.000	1650.00	888.82	1.42	1.83	.00	145.09	392.45
12460.000	2000.00	889.34	.51	1.77	.00	158.33	392.45
12460.000	2800.00	890.38	1.05	1.71	.00	191.60	392.45

SUMMARY OF WARNING AND STATUS MESSAGES :

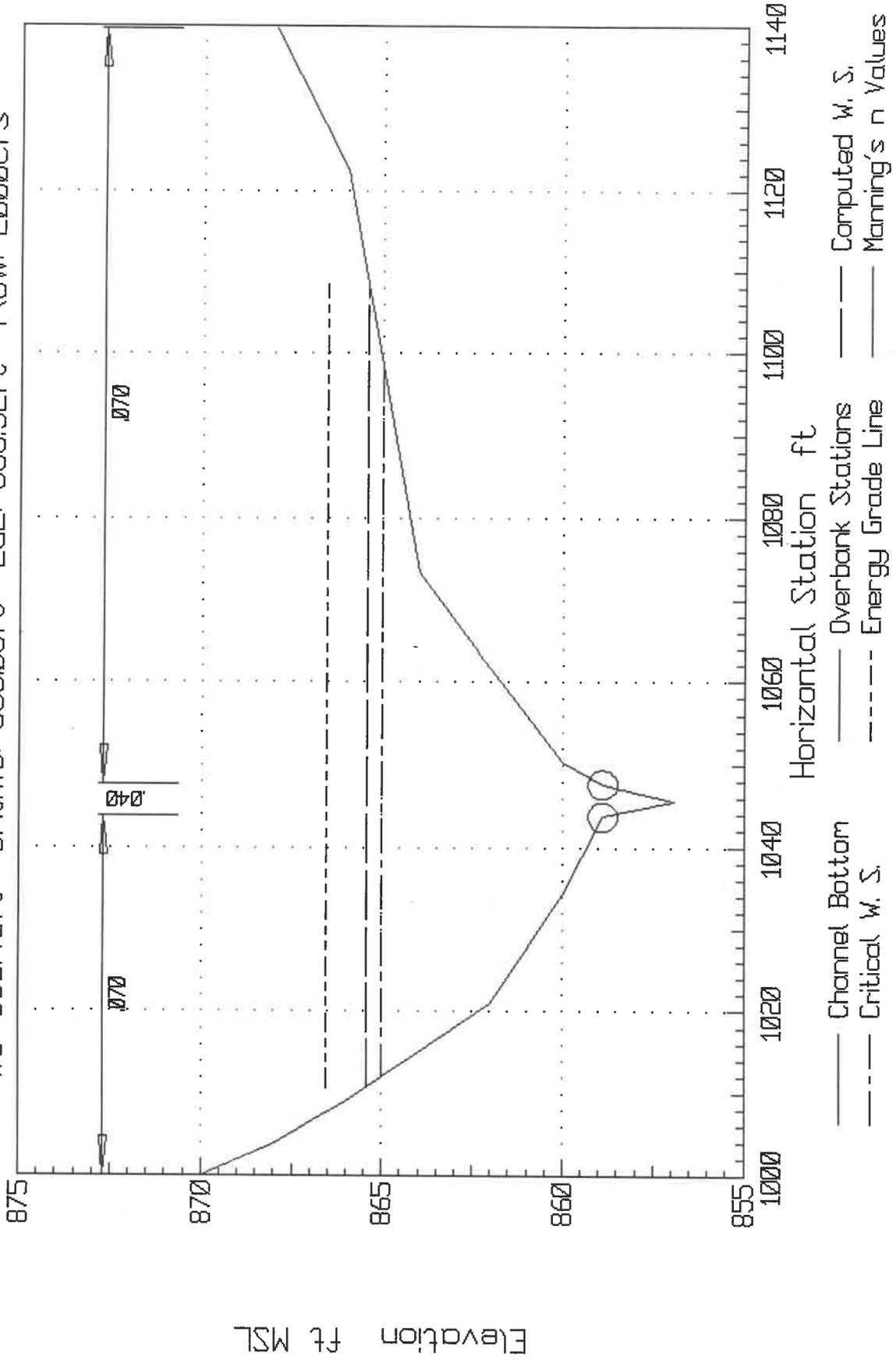
- Section 8989, profile 1, critical depth assumed.
- Section 8989, profile 1, minimum specific energy.
- Section 8989, profile 2, critical depth assumed.
- Section 8989, profile 2, minimum specific energy.
- Section 8989, profile 3, critical depth assumed.
- Section 8989, profile 3, minimum specific energy.
- Section 8989, profile 4, critical depth assumed.
- Section 8989, profile 4, minimum specific energy.
- Section 9256, profile 1, conveyance change outside acceptable range.

Section 9256, profile 2, conveyance change outside acceptable range.
Section 9256, profile 3, conveyance change outside acceptable range.
Section 9256, profile 4, conveyance change outside acceptable range.
Section 9536, profile 1, conveyance change outside acceptable range.
Section 9536, profile 2, conveyance change outside acceptable range.
Section 9536, profile 3, conveyance change outside acceptable range.
Section 9536, profile 4, conveyance change outside acceptable range.
Section 9576, profile 3, conveyance change outside acceptable range.
Section 9724, profile 1, conveyance change outside acceptable range.
Section 9724, profile 2, conveyance change outside acceptable range.
Section 9724, profile 3, conveyance change outside acceptable range.
Section 9724, profile 4, conveyance change outside acceptable range.
Section 10492, profile 1, conveyance change outside acceptable range.
Section 10492, profile 2, conveyance change outside acceptable range.
Section 10492, profile 3, conveyance change outside acceptable range.
Section 10492, profile 4, conveyance change outside acceptable range.
Section 10912, profile 1, conveyance change outside acceptable range.
Section 11619, profile 2, conveyance change outside acceptable range.
Section 11619, profile 3, conveyance change outside acceptable range.
Section 11619, profile 4, conveyance change outside acceptable range.
Section 12068, profile 1, conveyance change outside acceptable range.
Section 12068, profile 2, conveyance change outside acceptable range.
Section 12068, profile 3, conveyance change outside acceptable range.
Section 12068, profile 4, conveyance change outside acceptable range.

33 Warning and status message(s) generated

Cross-Section Plot 8638 ft

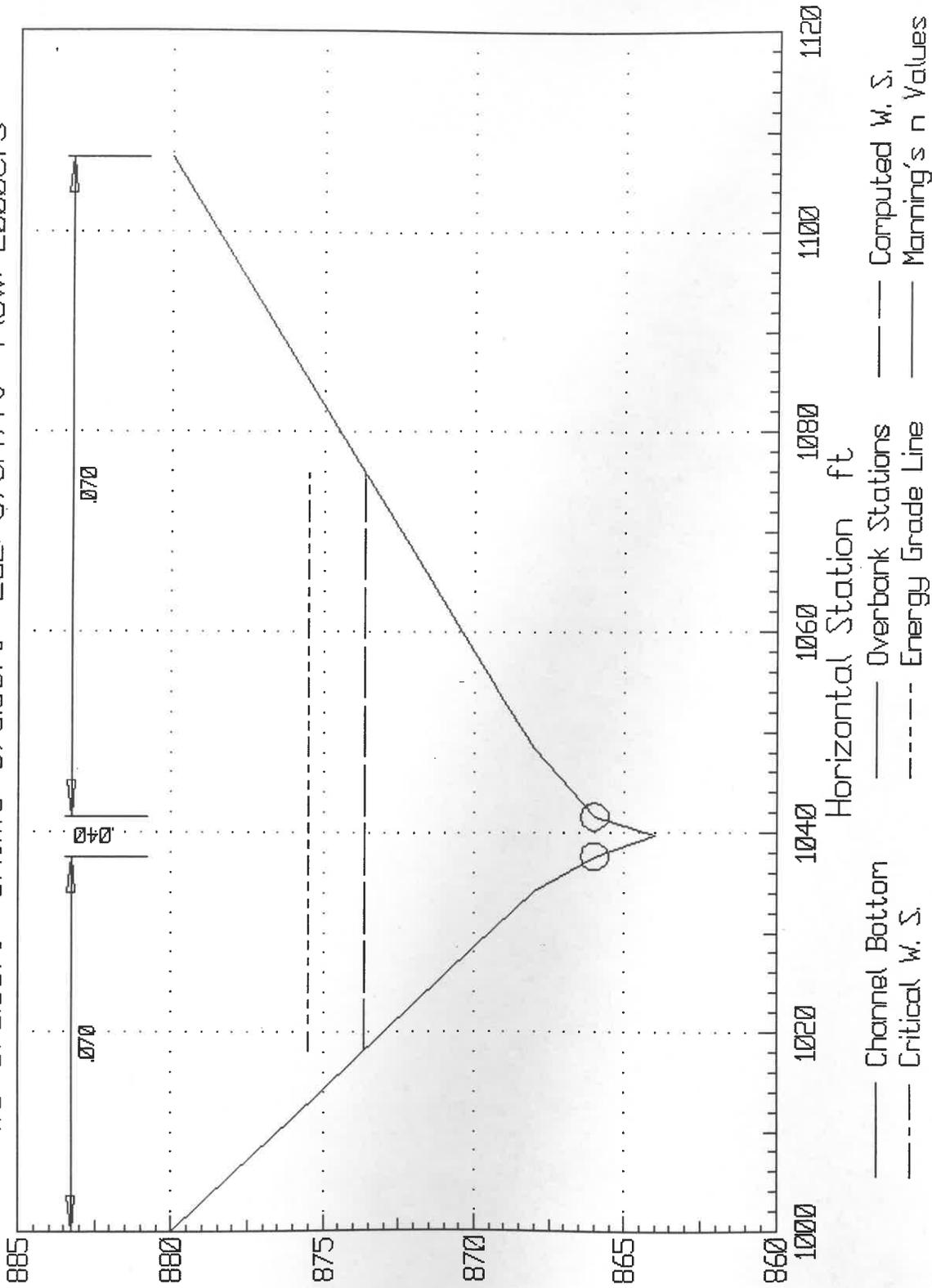
WS: 865.43ft Crit.WS: 865.03ft EGL: 866.52ft Flow: 2000cfs



Mann Valley
100-year flow

Cross-Section Plot 8989 ft

WS: 873.63ft Crit.WS: 873.63ft EGL: 875.47ft Flow: 2000cfs

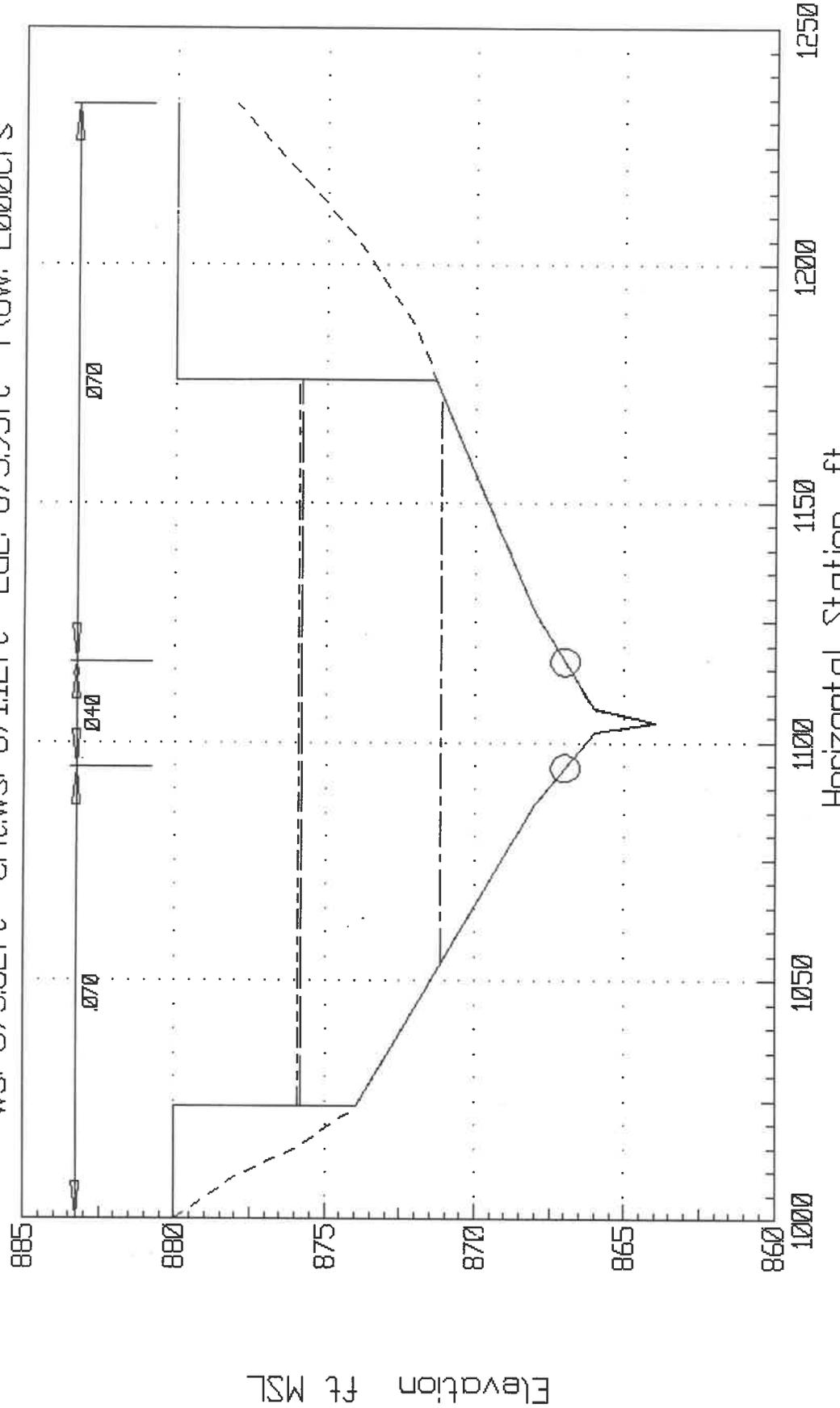


Mann Valley
100-year flow

- Channel Bottom
- Critical W. S.
- Overbank Stations
- Energy Grade Line
- Computed W. S.
- Manning's n Values

Cross-Section Plot 9256 ft

WS: 875.82ft Crit.WS: 871.12ft EGL: 875.93ft Flow: 2000cfs



Original Channel
 Computed W. S.
 Manning's n Values

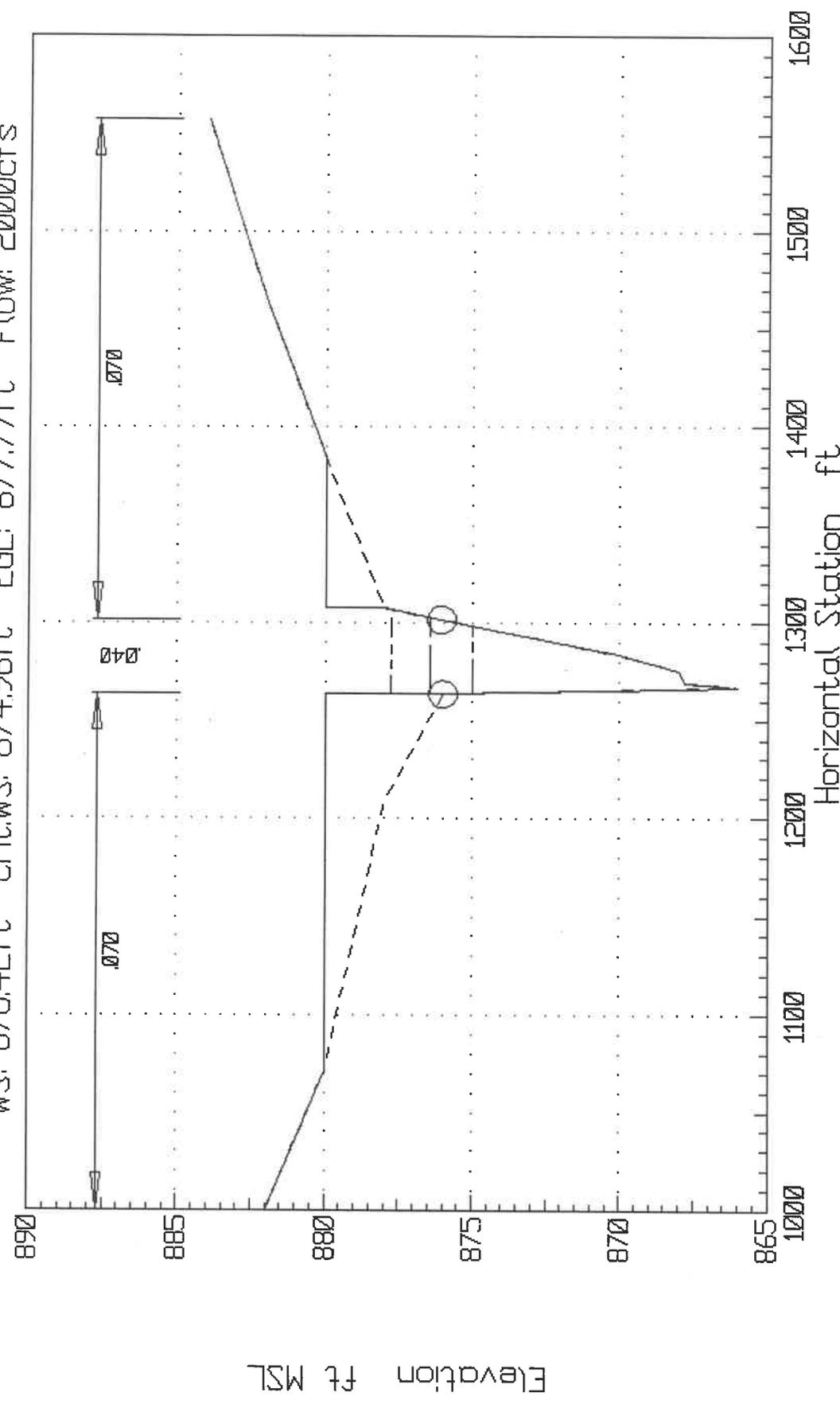
Revised Channel
 Critical W. S.

Overbank Stations
 Energy Grade Line

Mann Valley
 100-year flow

Cross-Section Plot 9536 ft

WS: 876.42ft Crit.WS: 874.98ft EGL: 877.77ft Flow: 20000cfs

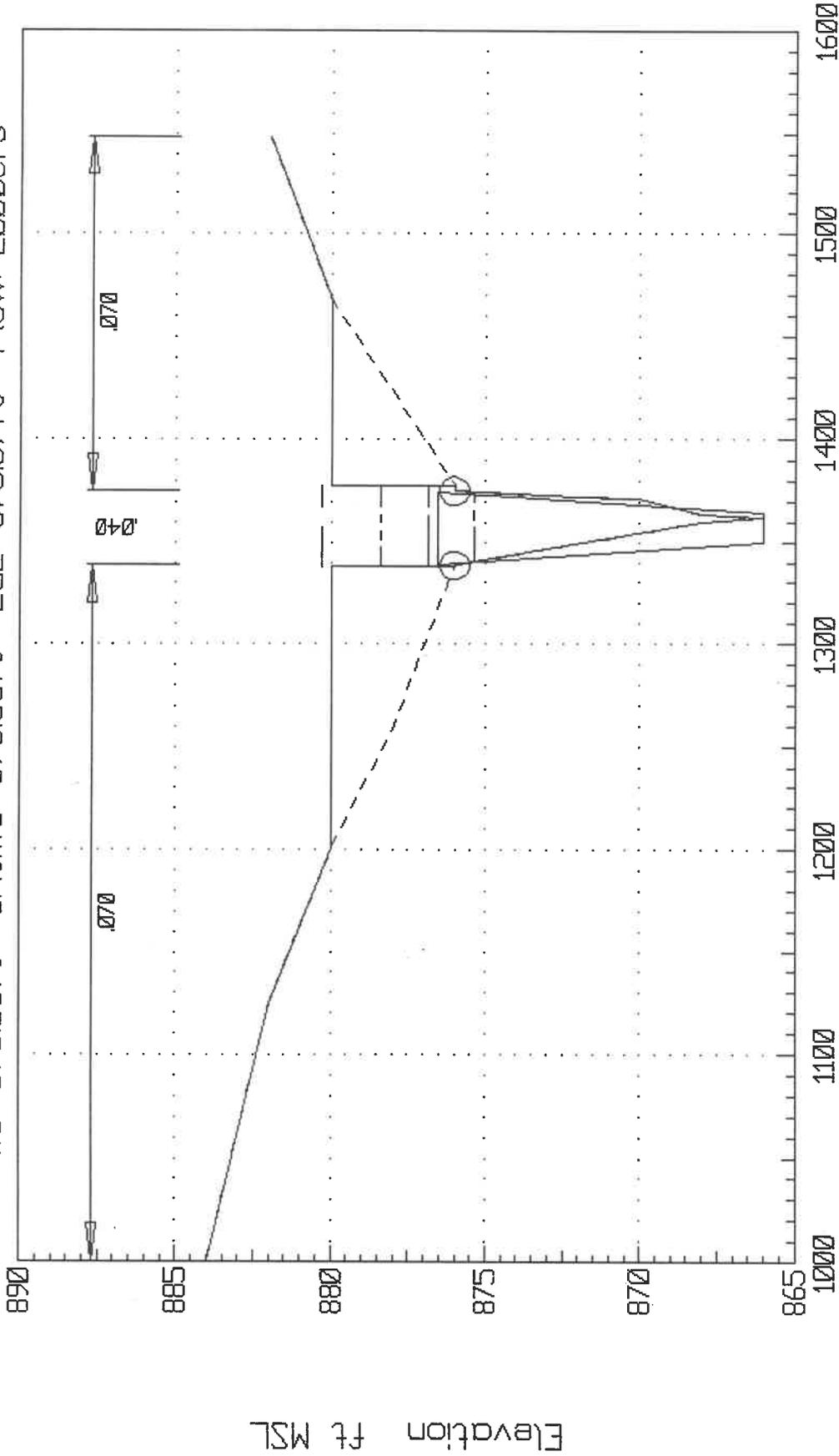


- Original Channel
- Computed W. S.
- Manning's n Values
- Revised Channel
- Critical W. S.
- Overbank Stations
- Energy Grade Line

Mann Valley
100-year flow

Bridge Cross-Section Plot 9576 ft

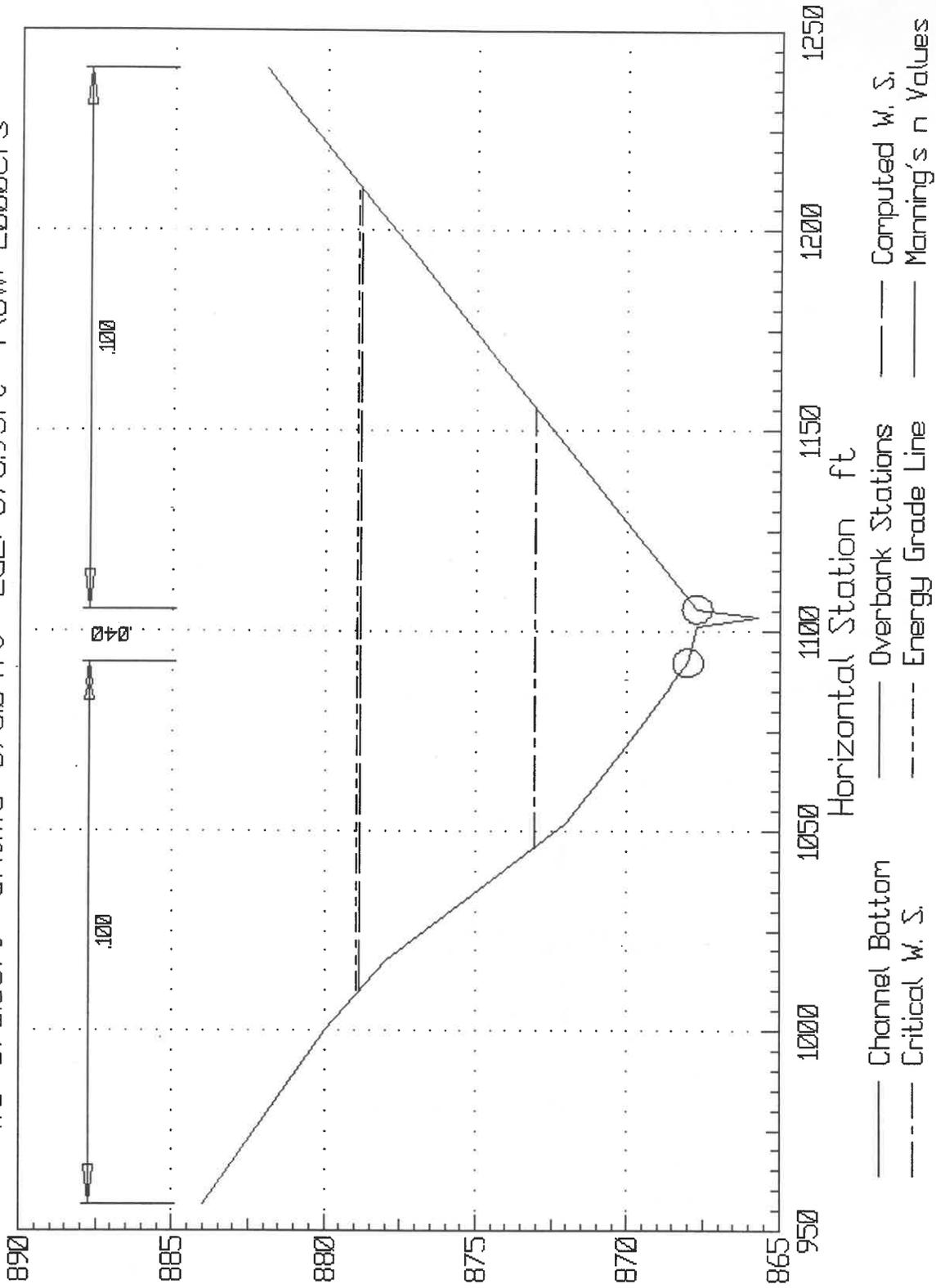
WS: 876.86ft Crit.WS: 875.36ft EGL: 878.37ft Flow: 2000cfs



- Original Channel
 - Bridge Low Chord
 - Computed W. S.
 - Manning's n Values
 - Revised Channel
 - Top of Roadway
 - Critical W. S.
 - Bridge Opening
 - Overbank Stations
 - Energy Grade Line
- Mann Valley**
100-year flow

Cross-Section Plot 9724 ft

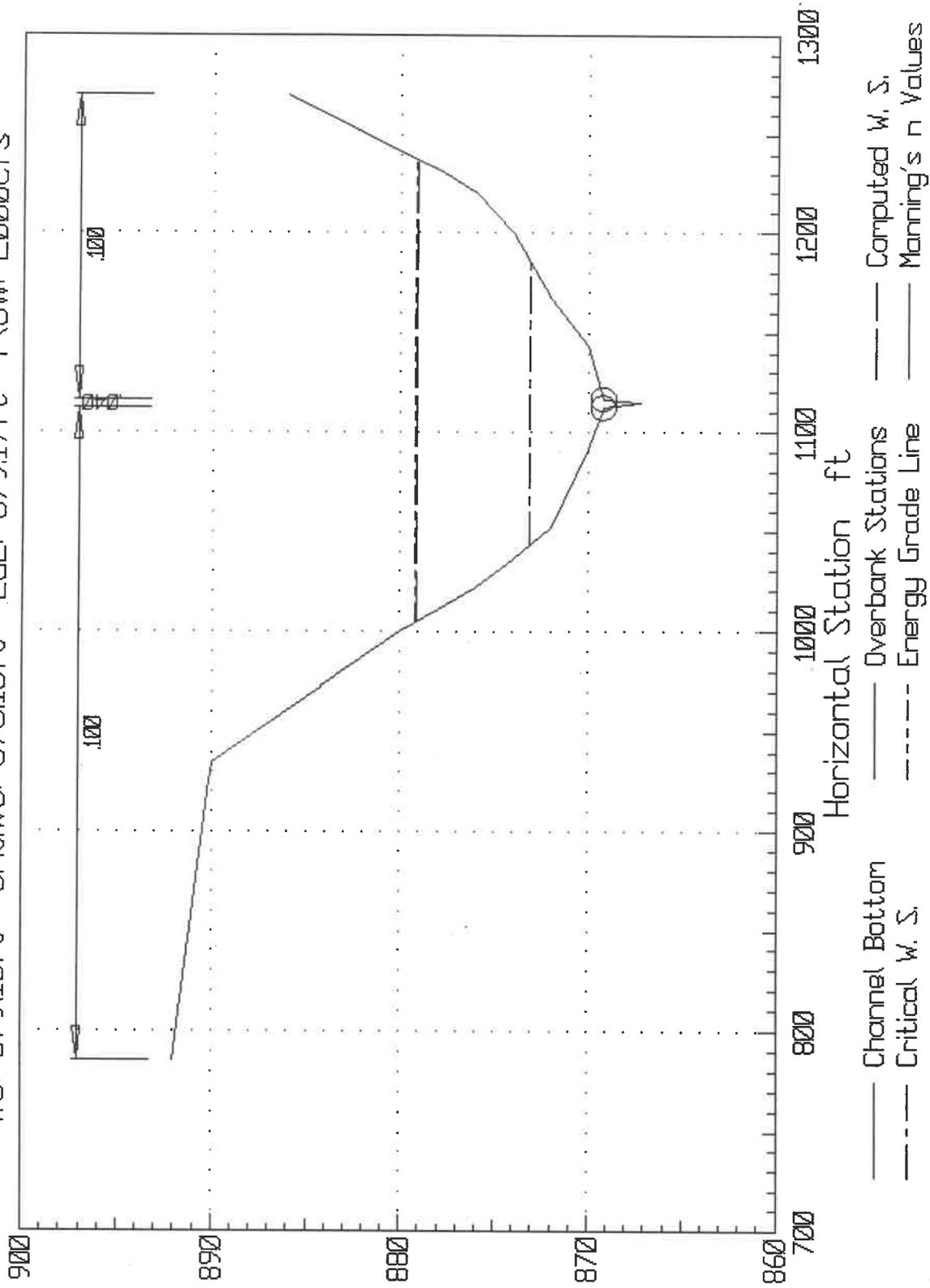
WS: 878.85ft Crit.WS: 873.04ft EGL: 878.95ft Flow: 2000cfs



Mann Valley
100-year flow

Cross-Section Plot 10125 ft

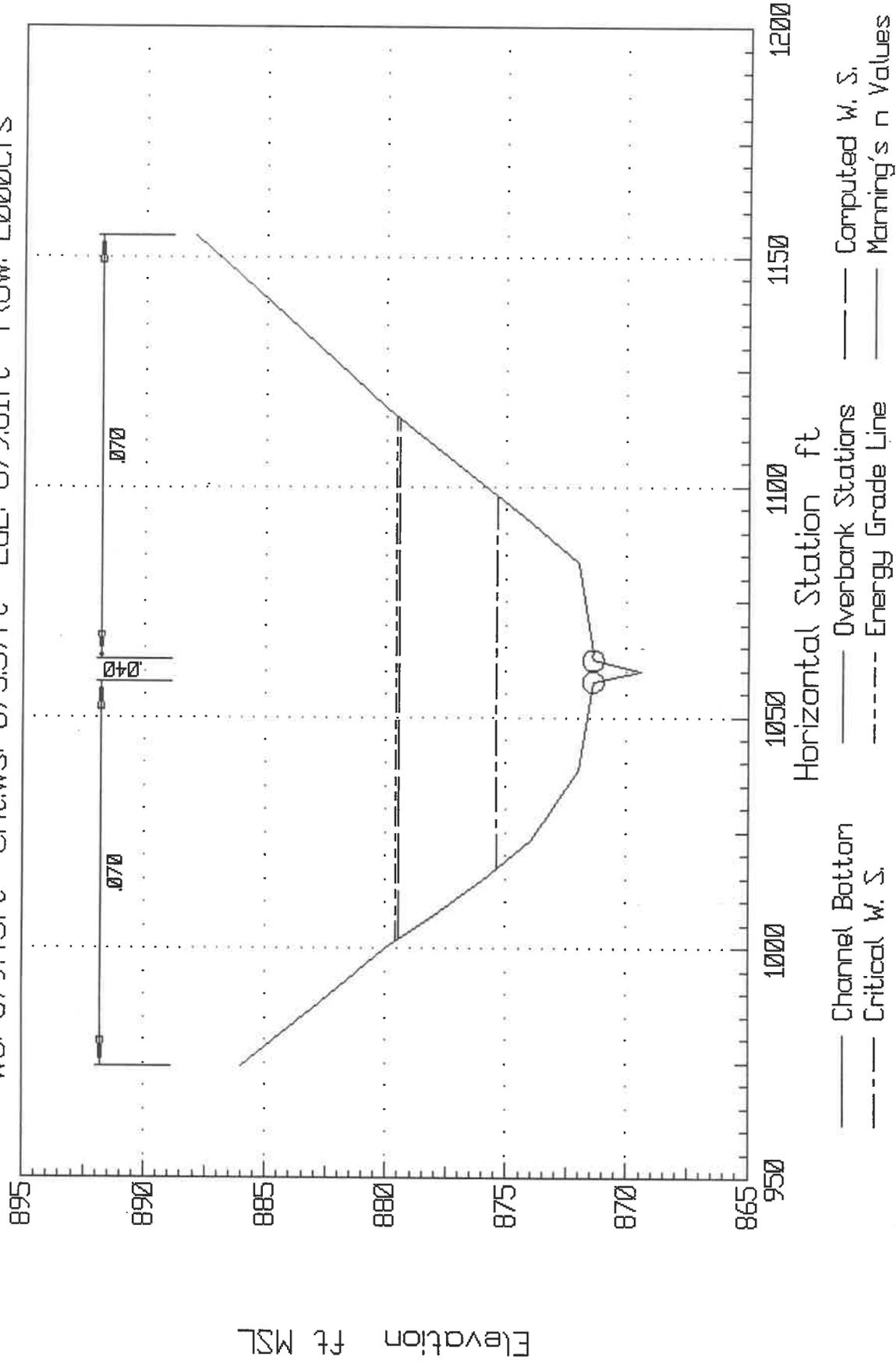
WS: 879.13ft Crit.WS: 873.15ft EGL: 879.17ft Flow: 2000cfs



Mann Valley
100-year flow

Cross-Section Plot 10492 ft

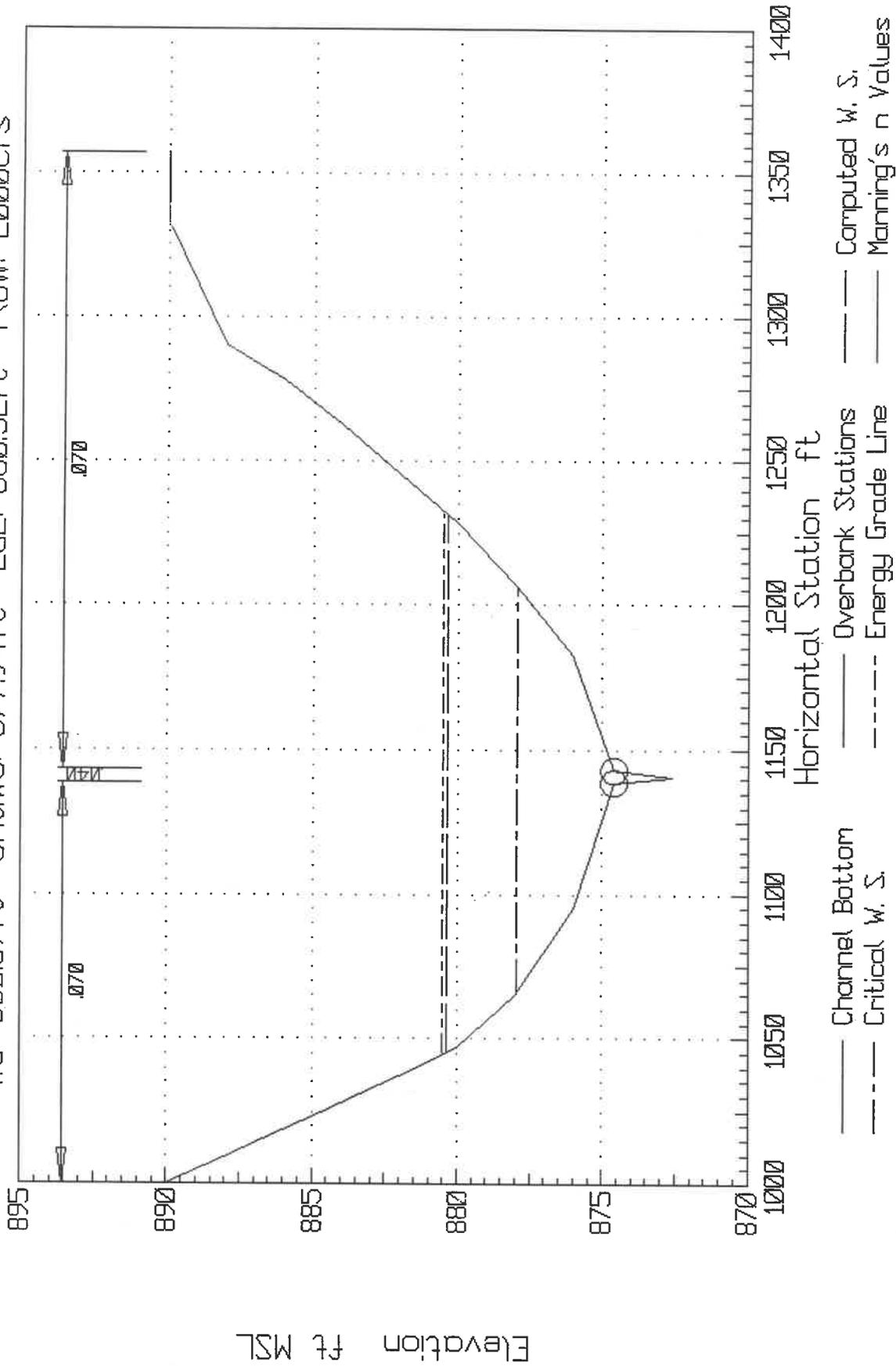
WS: 879.43ft Crit.WS: 875.37ft EGL: 879.61ft Flow: 20000cfs



Mann Valley
100-year flow

Cross-Section Plot 10912 ft

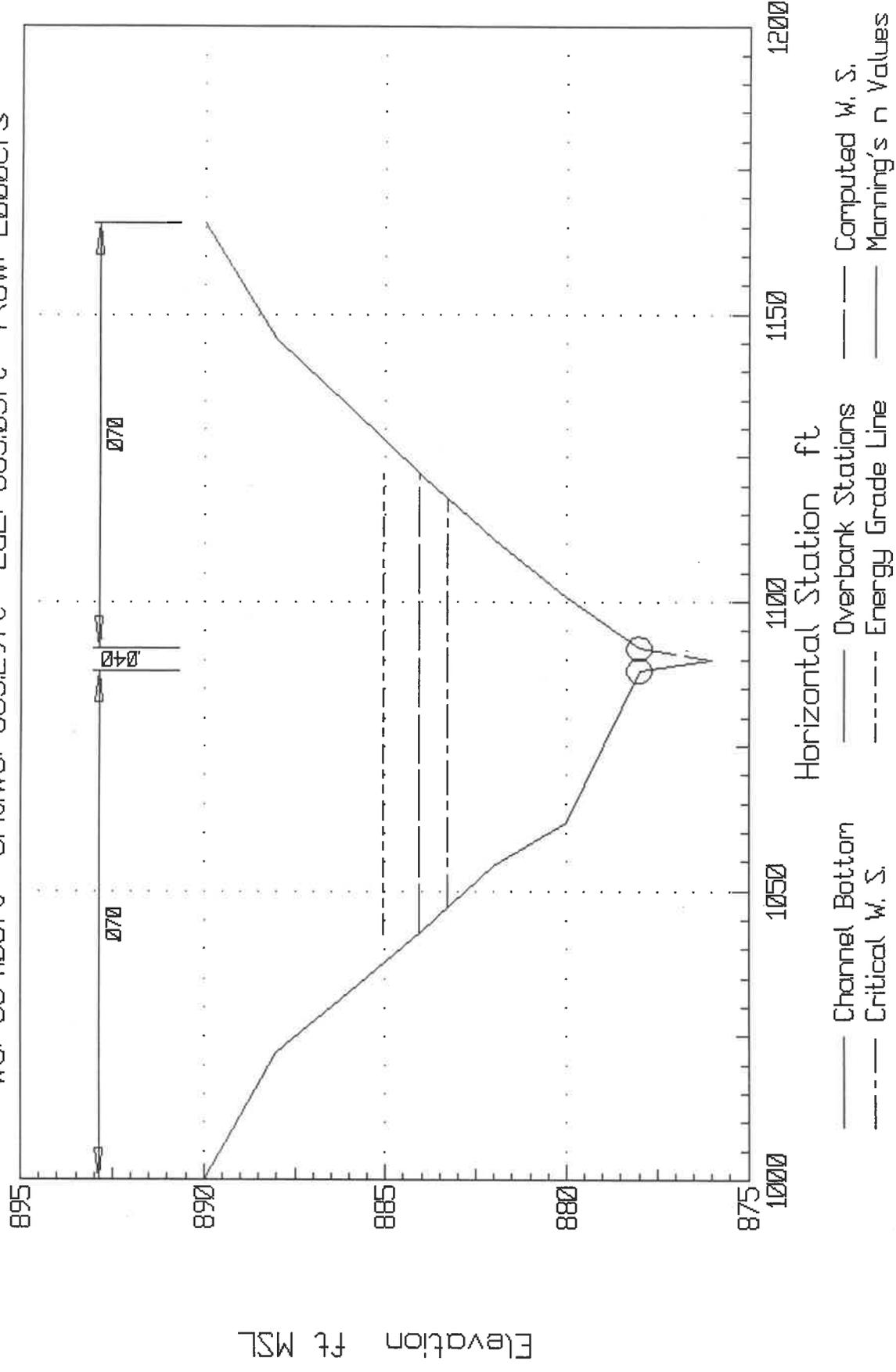
WS: 880.37ft Crit.WS: 877.94ft EGL: 880.52ft Flow: 2000cfs



Mann Valley
100-year flow

Cross-Section Plot 11619 ft

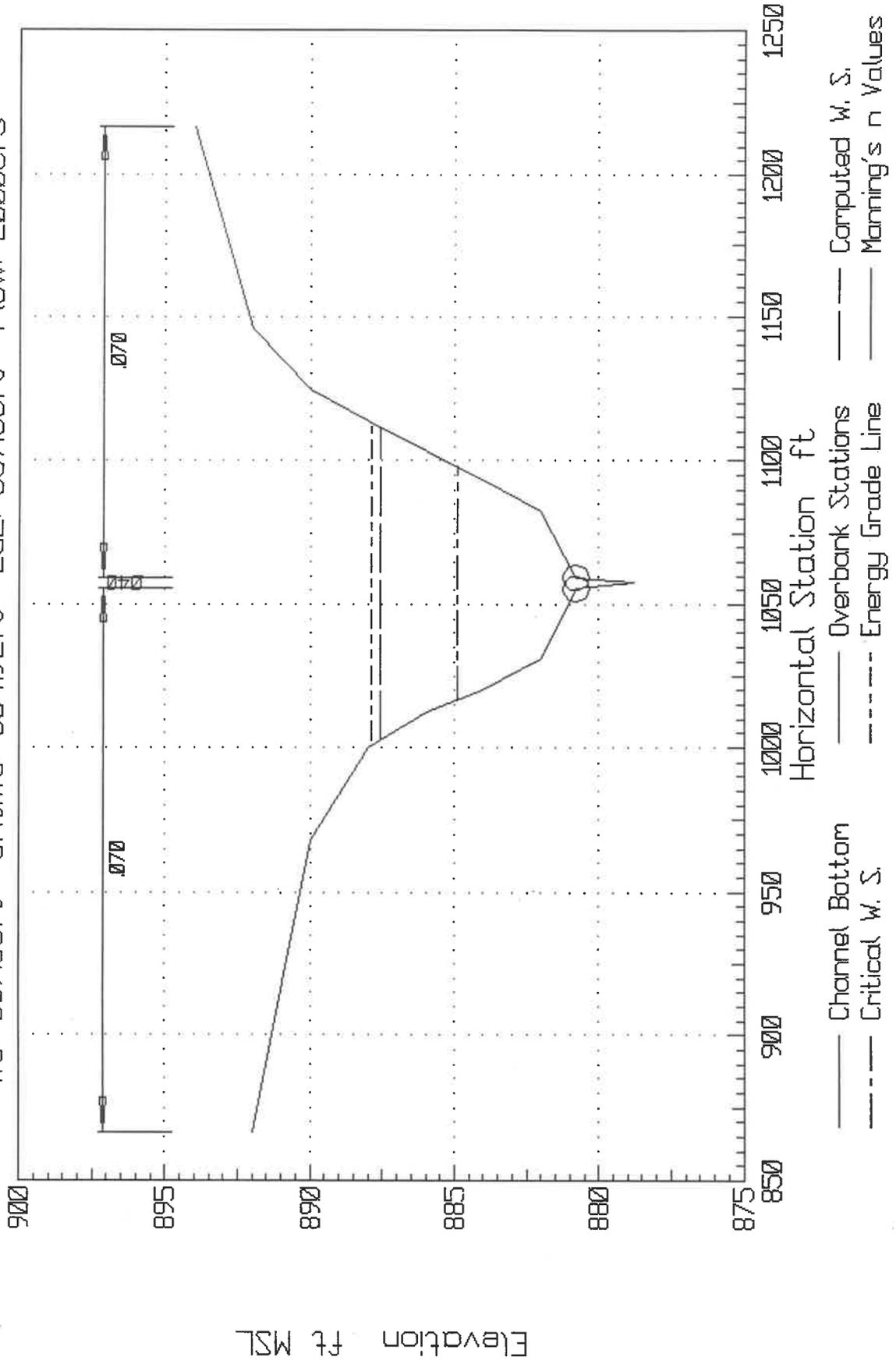
WS: 884.05ft Crit.WS: 883.29ft EGL: 885.03ft Flow: 2000cfs



Mann Valley
100-year flow

Cross-Section Plot 12068 ft

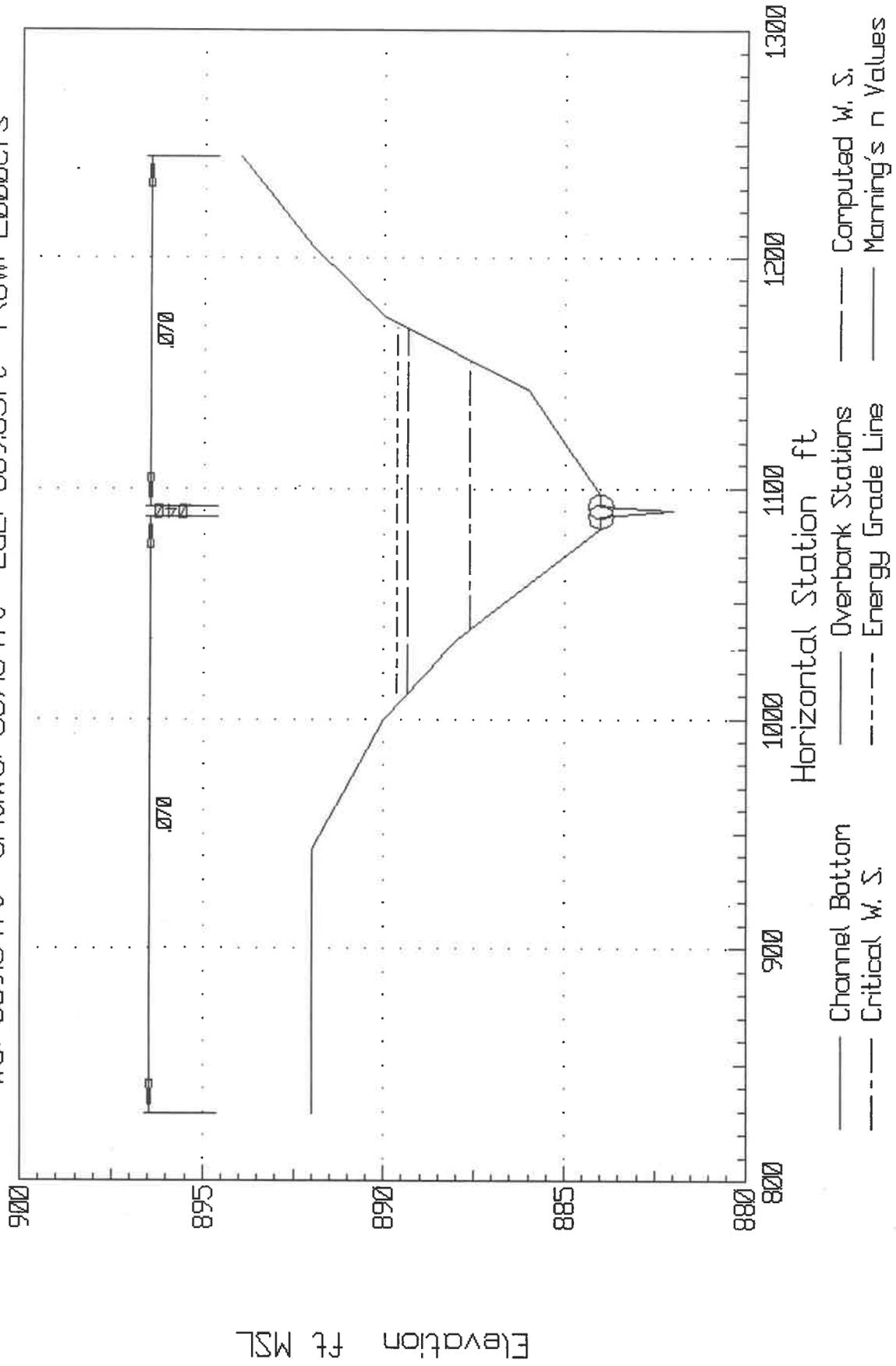
WS: 887.56ft Crit.WS: 884.92ft EGL: 887.86ft Flow: 2000cfs



Mann Valley
100-year flow

Cross-Section Plot 12460 ft

WS: 889.34ft Crit.WS: 887.64ft EGL: 889.63ft Flow: 2000cfs



Mann Valley
100-year flow

Mann Valley

Background Data for Hydraulic Model



Stream and location: MANN VALLEY ; 40 RIVER FALLS, WI
 Reach: MANU 2 ABOVE MM
 Event for which n is assigned: Q₁₀₀

- Is roughness uniform throughout the reach being considered? If not, n should be assigned for the average condition of the reach. *Broken into reaches of homogeneous*
- Is roughness uniformly distributed along the cross section? Is a division between channel and flood plain necessary? (Channel roughness uses steps 3–13, flood-plain roughness uses steps 14–23.)
 Is roughness uniformly distributed across the channel? If not, on what basis should n for the individual segments be weighted? *Division Needed*
- Describe the channel. Are present conditions representative of those during the flood? If not, describe the probable conditions during the flood.
- How will the roughness-producing effects of the following on the channel be accounted for?
 Bank roughness: _____
 Bedrock outcrops: _____
 Isolated boulders: _____
 Vegetation: _____
 Obstructions: _____
 Meander: _____

5–10. Computation of weighted n for the channel

Segment number and material	Approximate dimensions, (ft)		Wetted perimeter, (ft)	Area, (ft ²)	Median grain size, (mm)	Base n for segment	Adjustments	Adjusted n	Weight factor	Adjusted n times weight factor
	Width	Depth								
D. Soft mm	4	4	11.3	8	Sandy*	0.015				
just u.s. mm	4	4	11.3	8	Sandy	" "				
u.s. of mm	4	4	11.3	8	Sandy	" "				
								Sum		
								Weighted $n =$		

* Based upon soils map
 # Table 1

Figure 22. Sample form for computing n values (modified from Aldridge and Garrett, 1973, fig. 4).



11-13. Computation of n for the channel

Adjustment factors for the channel

Factor	Describe conditions briefly	Adjustment
Irregularity, n_1	Moderate → all	0.01
Alignment, n_2	occasional - all	0.001
Obstructions, n_3	MINOR → all	0.01
Vegetation, n_4	med	0.01
Meander, m	MINOR	1.00
Weighted n plus adjustments		.031
Computed $n =$.046

14. Describe the flood plain.

Are present conditions representative of those during the flood?

If not, describe probable conditions during the flood.

⇒ use 0.04

15. Is the roughness coefficient to be determined by roughness factors only or is it to include vegetation-density method?

16. Is roughness uniformly distributed across the flood plain?

If not, how should the flood plain be subdivided?

17-23. Computation of n for flood plain

Adjustment factors not using vegetation-density method

Subsection	Base n , n_b	Irregularity, n_1	Obstructions, n_3	Vegetation, n_4	Computed n
Below MM	0.015	slightly 0.005	MINOR 0.005	Large .05	.065 ⇒ use 0.05
Just above MM	0.01	0.005	0.005	very large 0.08	.105 ⇒ use 0.1
W/S BMM	0.01	0.005	0.005	Large	.065 ⇒ use 0.05

Adjustment factors using vegetation-density method

Sub-section	Base n , n_b	Irregularity, n_1	Obstructions, n_3	Vegetation, n_4	Boundary roughness, $n_0 = n_b + n_1 + n_3 + n_4$	Vegetation density, Veg_d	Effective drag, C_*	Hydraulic radius, R	Computed n

Figure 22. Sample form for computing n values (modified from Aldridge and Garrett, 1973, fig. 4)—Continued.

MP 0.1 ⇒ XS 1025

Table 3. Adjustment values for factors that affect roughness of flood plains

[Modified from Aldridge and Garrett, 1973, table 2]

Flood-plain conditions	n value adjustment	Example	
Degree of irregularity (n_1)	Smooth	0.000	Compares to the smoothest, flattest flood plain attainable in a given bed material.
	Minor	0.001–0.005	Is a flood plain slightly irregular in shape. A few rises and dips or sloughs may be visible on the flood plain.
	Moderate	0.006–0.010	Has more rises and dips. Sloughs and hummocks may occur.
	Severe	0.011–0.020	Flood plain very irregular in shape. Many rises and dips or sloughs are visible. Irregular ground surfaces in pastureland and furrows perpendicular to the flow are also included.
Variation of flood-plain cross section (n_2)	0.0	Not applicable.	
Effect of obstructions (n_3)	Negligible	0.000–0.004	Few scattered obstructions, which include debris deposits, stumps, exposed roots, logs, or isolated boulders, occupy less than 5 percent of the cross-sectional area.
	Minor	0.005–0.019	Obstructions occupy less than 15 percent of the cross-sectional area.
	Appreciable	0.020–0.030	Obstructions occupy from 15 to 50 percent of the cross-sectional area.
Amount of vegetation (n_4)	Small	0.001–0.010	Dense growth of flexible turf grass, such as Bermuda, or weeds growing where the average depth of flow is at least two times the height of the vegetation, or supple tree seedlings such as willow, cottonwood, arrowweed, or saltcedar growing where the average depth of flow is at least three times the height of the vegetation.
	Medium	0.011–0.025	Turf grass growing where the average depth of flow is from one to two times the height of the vegetation, or moderately dense stemmy grass, weeds, or tree seedlings growing where the average depth of flow is from two to three times the height of the vegetation; brushy, moderately dense vegetation, similar to 1- to 2-year-old willow trees in the dormant season.
	Large	0.025–0.050	Turf grass growing where the average depth of flow is about equal to the height of the vegetation, or 8- to 10-year-old willow or cottonwood trees intergrown with some weeds and brush (none of the vegetation in foliage) where the hydraulic radius exceeds 2 ft, or mature row crops such as small vegetables, or mature field crops where depth of flow is at least twice the height of the vegetation.
	Very large	0.050–0.100	Turf grass growing where the average depth of flow is less than half the height of the vegetation, or moderate to dense brush, or heavy stand of timber with few down trees and little undergrowth where depth of flow is below branches, or mature field crops where depth of flow is less than the height of the vegetation.
	Extreme	0.100–0.200	Dense bushy willow, mesquite, and saltcedar (all vegetation in full foliage), or heavy stand of timber, few down trees, depth of flow reaching branches.
Degree of meander (m)	1.0	Not applicable.	

Chow (1959) presents a table showing minimum, normal, and maximum values of n for flood plains covered by pasture and crops. These values are helpful for comparing the roughness values of flood plains having similar vegetation.

Vegetation-Density Method

For a wooded flood plain, the vegetation-density method can be used as an alternative to the previous method for determining n values for flood plains. In a wooded flood plain, where the tree diameters can be measured, the vegetation density of the flood plain can be determined.

Determining the vegetation density is an effective way of relating plant height and density characteristics, as a function of depth of flow, to the flow resistance of vegetation. Application of the flow-resistance model presented below requires an estimate of the vegetation density as a function of depth of flow. The procedure requires a direct or indirect determination of vegetation density at a given depth. If the change in n value through a range in depth is required, then an estimation of vegetation density through that range is necessary.

Techniques for Determining Vegetation Density

Petryk and Bosmajian (1975) developed a method of analysis of the vegetation density to determine the rough-

Table 2. Adjustment values for factors that affect the roughness of a channel

[Modified from Aldridge and Garrett, 1973, table 2]

Channel conditions		<i>n</i> value adjustment ¹	Example
Degree of irregularity (<i>n</i> ₁)	Smooth	0.000	Compares to the smoothest channel attainable in a given bed material.
	Minor	0.001–0.005	Compares to carefully dredged channels in good condition but having slightly eroded or scoured side slopes.
	Moderate	0.006–0.010	Compares to dredged channels having moderate to considerable bed roughness and moderately sloughed or eroded side slopes.
	Severe	0.011–0.020	Badly sloughed or scalloped banks of natural streams; badly eroded or sloughed sides of canals or drainage channels; unshaped, jagged, and irregular surfaces of channels in rock.
Variation in channel cross section (<i>n</i> ₂)	Gradual	0.000	Size and shape of channel cross sections change gradually.
	Alternating occasionally	0.001–0.005	Large and small cross sections alternate occasionally, or the main flow occasionally shifts from side to side owing to changes in cross-sectional shape.
	Alternating frequently	0.010–0.015	Large and small cross sections alternate frequently, or the main flow frequently shifts from side to side owing to changes in cross-sectional shape.
Effect of obstruction (<i>n</i> ₃)	Negligible	0.000–0.004	A few scattered obstructions, which include debris deposits, stumps, exposed roots, logs, piers, or isolated boulders, that occupy less than 5 percent of the cross-sectional area.
	Minor	0.005–0.015	Obstructions occupy less than 15 percent of the cross-sectional area, and the spacing between obstructions is such that the sphere of influence around one obstruction does not extend to the sphere of influence around another obstruction. Smaller adjustments are used for curved smooth-surfaced objects than are used for sharp-edged angular objects.
	Appreciable	0.020–0.030	Obstructions occupy from 15 to 50 percent of the cross-sectional area, or the space between obstructions is small enough to cause the effects of several obstructions to be additive, thereby blocking an equivalent part of a cross section.
	Severe	0.040–0.050	Obstructions occupy more than 50 percent of the cross-sectional area, or the space between obstructions is small enough to cause turbulence across most of the cross section.
Amount of vegetation (<i>n</i> ₄)	Small	0.002–0.010	Dense growths of flexible turf grass, such as Bermuda, or weeds growing where the average depth of flow is at least two times the height of the vegetation; supple tree seedlings such as willow, cottonwood, arrowweed, or saltcedar growing where the average depth of flow is at least three times the height of the vegetation.
	Medium	0.010–0.025	Turf grass growing where the average depth of flow is from one to two times the height of the vegetation; moderately dense stemmy grass, weeds, or tree seedlings growing where the average depth of flow is from two to three times the height of the vegetation; brushy, moderately dense vegetation, similar to 1- to 2-year-old willow trees in the dormant season, growing along the banks, and no significant vegetation is evident along the channel bottoms where the hydraulic radius exceeds 2 ft.
	Large	0.025–0.050	Turf grass growing where the average depth of flow is about equal to the height of the vegetation; 8- to 10-year-old willow or cottonwood trees intergrown with some weeds and brush (none of the vegetation in foliage) where the hydraulic radius exceeds 2 ft; bushy willows about 1 year old intergrown with some weeds along side slopes (all vegetation in full foliage), and no significant vegetation exists along channel bottoms where the hydraulic radius is greater than 2 ft.
	Very large	0.050–0.100	Turf grass growing where the average depth of flow is less than half the height of the vegetation; bushy willow trees about 1 year old intergrown with weeds along side slopes (all vegetation in full foliage), or dense cattails growing along channel bottom; trees intergrown with weeds and brush (all vegetation in full foliage).
Degree of meandering ² (<i>m</i>)	Minor	1.00	Ratio of the channel length to valley length is 1.0 to 1.2.
	Appreciable	1.15	Ratio of the channel length to valley length is 1.2 to 1.5.
	Severe	1.30	Ratio of the channel length to valley length is greater than 1.5.

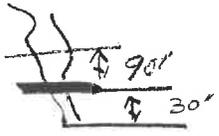
¹ Adjustments for degree of irregularity, variations in cross section, effect of obstructions, and vegetation are added to the base *n* value (table 1) before multiplying by the adjustment for meander.

² Adjustment values apply to flow confined in the channel and do not apply where downvalley flow crosses meanders.

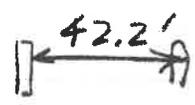
FROM WIS PRO COUNTY HWY M MODEL

CR M BRIDGE

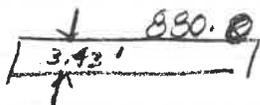
modeled reach:
 30' downstream
 90' upstream



BL Low Chord = 876.55
 Length of Bridge between abutments =
 BRIDGE centered @ station 140



BD Depth of bridge deck = 3.43'
 Elevation of the top of the deck



CD Sloping embankment and sloping spill through abutments.
 Total Bridge width = 40'
 Embankment Sideslope = 3:1
 Embankment Elevation = 880

*	MANNINGS N
N	0.040
*	
XS	FULV 530 * * * 0.006
*	X-SECTION AT BRIDGE
BR	BR1 530 876.55
BL	0 42.2 138 142 ✓
BD	3.43 880 ✓
CD	3 40 3 880 ✓
AB	1.5
N	0.045

AB Slope Horizontal = 1.5' / 1'

Q 100YR.
 * 700
 SK 0.006 - FOR INITIAL WS ~ energy grade slope (ft/ft)
 *
 * Name

X-SECTION STARTS AT STA 8+50 (ON PLAN AND PROFILE)

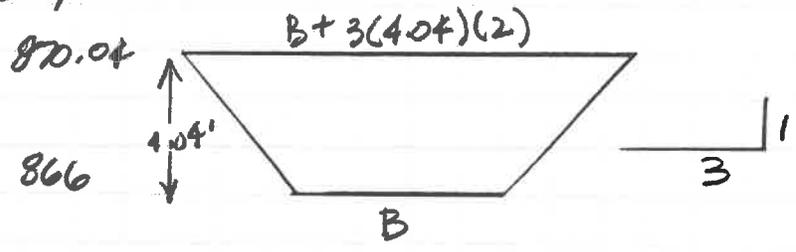
XS	EXIT 500
GR	0.880 28,878.00 44,876 53,874 75,872 101,870
GR	121,868 133,866 140,865.18 146,866 152,868
GR	167,870 180,872 190,874 205,874 234,872 280,872 320,880
SA	



C.T.H. MM BRIDGE

FROM WSPRO OUTPUT:

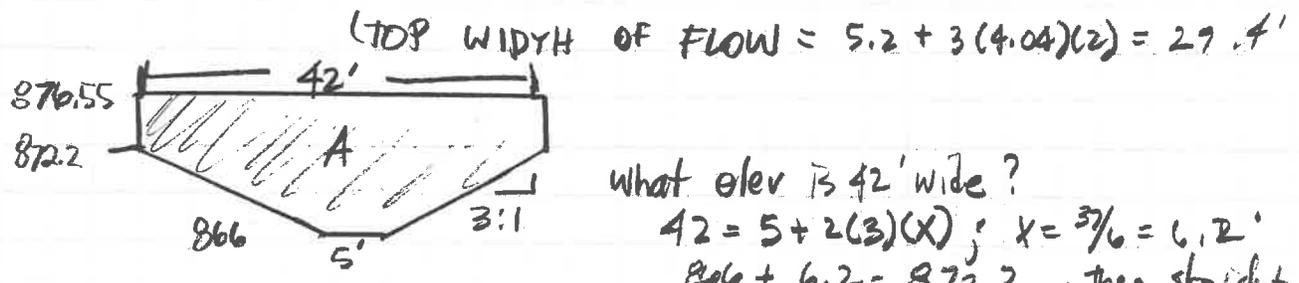
ARSA @ 870.04 = 70 sf
 BOTTOM ELEV = 866
 SIDESLOPES = 3:1



$$ARSA = 70 \text{ sf} = \frac{B + \{B + 3(4.04)(2)\}}{2} (4.04)$$

$$\frac{70(2)}{4.04} = 2B + 6(4.04)$$

$$\frac{1}{2} \left\{ \frac{70(2)}{4.04} - 6(4.04) \right\} = B = 5.2'$$



What elev is 42' wide?
 $42 = 5 + 2(3)(X)$; $X = \frac{37}{6} = 6.2'$
 $866 + 6.2 = 872.2$... then straight

$$AVAIL AREA = (876.55 - 872.2)(42') + \frac{(5 + 42)}{2} (872.2 - 866)$$

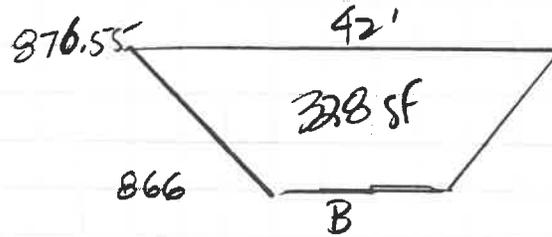
$$= 183 + 145 = 328 \text{ sf available} \Rightarrow \text{USE } 300 \text{ sf}$$

WSPRO Avail modeled would be more based upon side slope approx... (account for add'l riprap, etc which may be present @ abutments)

TRAPEZOIDAL APPROXIMATION

$A = 328 \text{ sf}$

TOP = 42'
 BOTTOM = ?



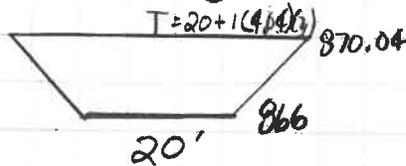
$$\left[\frac{(B+42)}{2} \right] (876.55 - 866) = 328$$

$$\frac{(B+42)}{2} = \frac{328}{10.55} = 31$$

$$B = (31)2 - 42 = 20'$$

SIDESLOPE = $\frac{42 - 20}{2(876.55 - 866)} = \frac{22}{2(10.55)} = 55 = 1.04 \Rightarrow 1:1$

Check area @ 876.04



$$A = \frac{20 + (20 + 1(4.04)(2))}{2} (4.04)$$

$$A = \frac{20 + 28}{2} (4.04) = 97 \text{ sf}$$

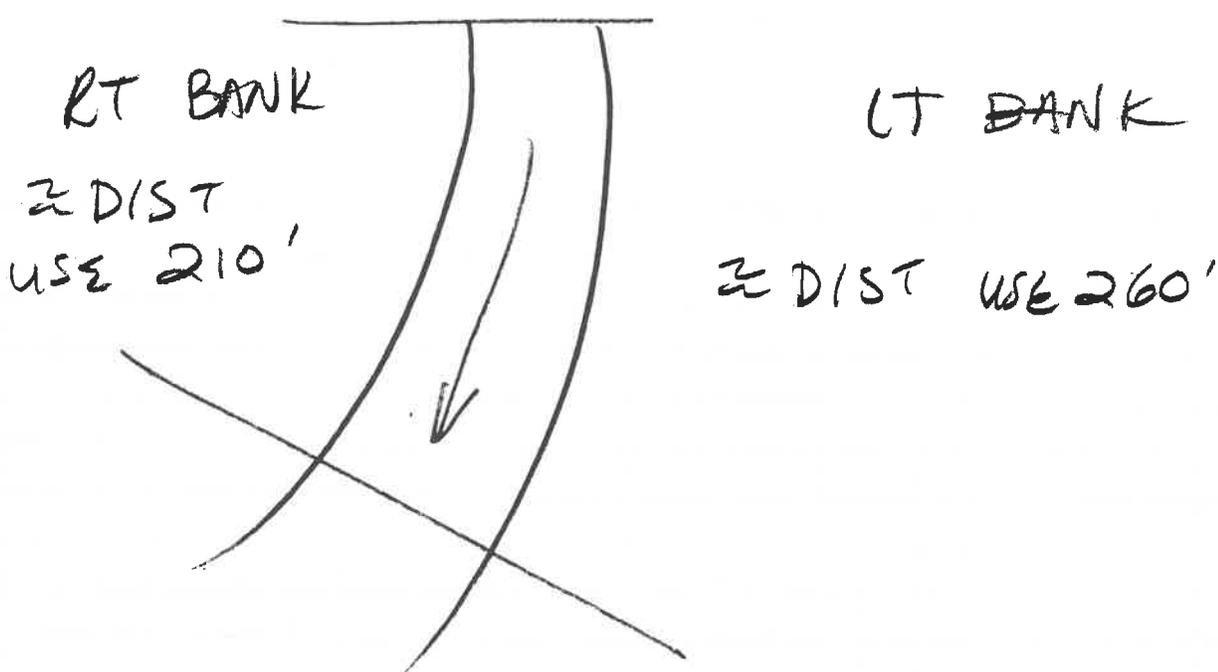
NOTE: THIS OVERESTIMATES AREA...

USE 15' BW w/ 1:1 $A_{(870.04)} = \frac{15 + (15 + 1(4.04)(2))}{2} (4.04) = 77 \text{ sf}$
 (w/in 10% OK)

total then is $A_{(876.55)} = \frac{15 + (15 + 1(876.55 - 866)(2))}{2} (876.55 - 866) = 270 \text{ sf}$

w/ 18% of approximated area. - ok - we.

DOWNSTREAM ENCROACHMENT (4:1 spread)



THEN RIGHT = $210/4 \approx 50'$ RT OF RT BANK
 LEFT = $260/4 \approx 65'$ LT OF LT BANK

RT BANK STA = USE 1126.94
 LT BANK STA = USE 1086.89

SO encroach @ RT $1126.94 + 50 \Rightarrow 1177$ ^{create} use 1176
 LT $1086.89 - 65 \Rightarrow 1022 \Rightarrow$ use 1023.33

~~UPSTREAM ENCROACHMENT NOT NEEDED DUE TO TOPOGRAPHY~~

T1
T2
T3
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J1
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J3
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SK
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XS
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EX
ER

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
P-55-074 MANN VALLEY CREEK
EXISTING CONDITIONS

1

SDR	Q	VEL	XSTW	YMIN	FR#	WSEL	EGL
6	5	13	28	23	14	3	7

100YR.
700

0.006

X-SECTION STARTS AT STA 8+50 (ON PLAN AND PROFILE)

EXIT	500
0,880	28,878.00
44,876	53,874
75,872	101,870
121,868	133,866
140,865.18	146,866
152,868	167,870
180,872	190,874
205,874	234,872
280,872	320,880

MANNINGS N
0.040

FULV 530 * * * 0.006

X-SECTION AT BRIDGE

BR1	530	877.40	15
131,877.4	131,866.18	140,865.36	146,866.18
153,866.5	153,877.4	131,877.4	
2	30	2	880
45			

0.045

APPR 590

0,880	13,874	50,872	95,870	103,868	138,868
143,866	145,865.72	148,866	165,868	175,870	
188,872	208,874	220,876	232,878	240,880	

MANNINGS N
0.040

1

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
 P060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
 (Input modified to free format by GKY&A 01/92)

*** RUN DATE & TIME: 02-05-96 09:20

T1 CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
 T2 P-55-074 MANN VALLEY CREEK
 T3 EXISTING CONDITIONS
 *
 *
 J1 1

J1 RECORD PARAMETERS:

DELTA Y = 1.00 YTOL = .02 QTOL = .02 FNTEST = .80 IHFNOJ = -1

*
 * SDR Q VEL XSTW YMIN FR# WSEL EGL
 J3 6 5 13 28 23 14 3 7
 *
 *

* 100YR.
 Q 700

*** Q-DATA FOR SEC-ID, ISEQ = 1

* SK 0.006

* X-SECTION STARTS AT STA 8+50 (ON PLAN AND PROFILE)

1

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
 P060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
 (Input modified to free format by GKY&A 01/92)

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
 P-55-074 MANN VALLEY CREEK
 EXISTING CONDITIONS

*** RUN DATE & TIME: 02-05-96 09:20

*** START PROCESSING CROSS SECTION - "EXIT "

XS EXIT 500
 GR 0,880 28,878.00 44,876 53,874 75,872 101,870
 GR 121,868 133,866 140,865.18 146,866 152,868
 GR 167,870 180,872 190,874 205,874 234,872 280,872 320,880
 SA
 * MANNINGS N
 N 0.040
 *

*** FINISH PROCESSING CROSS SECTION - "EXIT "

*** CROSS SECTION "EXIT " WRITTEN TO DISK, RECORD NO. = 1

--- DATA SUMMARY FOR SECID "EXIT " AT SRD = 500. ERR-CODE = 0

SKEW IHFNO VSLOPE EK CK
 .0 0. ***** .50 .00

X-Y COORDINATE PAIRS (NGP = 18):

X	Y	X	Y	X	Y	X	Y
.0	880.00	28.0	878.00	44.0	876.00	53.0	874.00

75.0	872.00	101.0	870.00	121.0	868.00	133.0	866.00
140.0	865.18	146.0	866.00	152.0	868.00	167.0	870.00
180.0	872.00	190.0	874.00	205.0	874.00	234.0	872.00
280.0	872.00	320.0	880.00				

X-Y MAX-MIN POINTS:

XMIN	Y	X	YMIN	XMAX	Y	X	YMAX
.0	880.00	140.0	865.18	320.0	880.00	.0	880.00

ROUGHNESS COEFFICIENTS (NSA = 1):
.040

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
P060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
P-55-074 MANN VALLEY CREEK
EXISTING CONDITIONS

*** RUN DATE & TIME: 02-05-96 09:20

*** START PROCESSING CROSS SECTION - "FULV "

XS FULV 530 * * * 0.006
* X-SECTION AT BRIDGE

*** FINISH PROCESSING CROSS SECTION - "FULV "

*** NO ROUGHNESS DATA INPUT, WILL PROPAGATE FROM PREVIOUS CROSS SECTION.
*** CROSS SECTION "FULV " WRITTEN TO DISK, RECORD NO. = 2

--- DATA SUMMARY FOR SECID "FULV " AT SRD = 530. ERR-CODE = 0

SKEW	IHFNO	VSLOPE	EK	CK
.0	0.	.0060	.50	.00

X-Y COORDINATE PAIRS (NGP = 18):

X	Y	X	Y	X	Y	X	Y
.0	880.18	28.0	878.18	44.0	876.18	53.0	874.18
75.0	872.18	101.0	870.18	121.0	868.18	133.0	866.18
140.0	865.36	146.0	866.18	152.0	868.18	167.0	870.18
180.0	872.18	190.0	874.18	205.0	874.18	234.0	872.18
280.0	872.18	320.0	880.18				

X-Y MAX-MIN POINTS:

XMIN	Y	X	YMIN	XMAX	Y	X	YMAX
.0	880.18	140.0	865.36	320.0	880.18	.0	880.18

ROUGHNESS COEFFICIENTS (NSA = 1):
.040

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
P060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
P-55-074 MANN VALLEY CREEK
EXISTING CONDITIONS

*** RUN DATE & TIME: 02-05-96 09:20

*** START PROCESSING CROSS SECTION - "BR1 "

BR BR1 530 877.40 15

GR 131,877.4 131,866.18 140,865.36 146,866.18
GR 153,866.5 153,877.4 131,877.4
CD 2 30 2 880 45
N 0.045
*
*

*** FINISH PROCESSING CROSS SECTION - "BR1 "
*** CROSS SECTION "BR1 " WRITTEN TO DISK, RECORD NO. = 3

--- DATA SUMMARY FOR SECID "BR1 " AT SRD = 530. ERR-CODE = 0

SKEW	IHFNO	VSLOPE	EK	CK
15.0	0.	.0060	.50	.00

X-Y COORDINATE PAIRS (NGP = 7):

X	Y	X	Y	X	Y	X	Y
131.0	877.40	131.0	866.18	140.0	865.36	146.0	866.18
153.0	866.50	153.0	877.40	131.0	877.40		

X-Y MAX-MIN POINTS:

XMIN	Y	X	YMIN	XMAX	Y	X	YMAX
131.0	877.40	140.0	865.36	153.0	866.50	131.0	877.40

ROUGHNESS COEFFICIENTS (NSA = 1):
.045

BRIDGE PARAMETERS:

BRTYPE	BRWDTH	LSEL	USERCD	EMBSS	EMBELV	YABLT	YABRT
2	30.0	877.40	*****	2.00	880.00	*****	*****

PIER DATA: NPW = 0 PPCD = **

1
WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
P060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
P-55-074 MANN VALLEY CREEK
EXISTING CONDITIONS

*** RUN DATE & TIME: 02-05-96 09:20

*** START PROCESSING CROSS SECTION - "APPR "

AS APPR 590
GR 0,880 13,874 50,872 95,870 103,868 138,868
GR 143,866 145,865.72 148,866 165,868 175,870
GR 188,872 208,874 220,876 232,878 240,880
SA
* MANNINGS N
N 0.040
*
EX

*** FINISH PROCESSING CROSS SECTION - "APPR "
*** CROSS SECTION "APPR " WRITTEN TO DISK, RECORD NO. = 4

--- DATA SUMMARY FOR SECID "APPR " AT SRD = 590. ERR-CODE = 0

SKEW	IHFNO	VSLOPE	EK	CK
.0	0.	.0060	.50	.00

X-Y COORDINATE PAIRS (NGP = 16):

X	Y	X	Y	X	Y	X	Y
.0	880.00	13.0	874.00	50.0	872.00	95.0	870.00
103.0	868.00	138.0	868.00	143.0	866.00	145.0	865.72
148.0	866.00	165.0	868.00	175.0	870.00	188.0	872.00
208.0	874.00	220.0	876.00	232.0	878.00	240.0	880.00

X-Y MAX-MIN POINTS:

XMIN	Y	X	YMIN	XMAX	Y	X	YMAX
.0	880.00	145.0	865.72	240.0	880.00	.0	880.00

ROUGHNESS COEFFICIENTS (NSA = 1):

.040

BRIDGE PROJECTION DATA: XREFLT XREFRT FDSTLT FDSTRT

+++ BEGINNING PROFILE CALCULATIONS -- 1

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
 P060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
 (Input modified to free format by GKY&A 01/92)

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
 P-55-074 MANN VALLEY CREEK
 EXISTING CONDITIONS

*** RUN DATE & TIME: 02-05-96 09:20

XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	
EXIT :XS	*****	101.	144.	.37	*****	870.34	869.04	700.	869.97
500.	*****	167.	9033.	1.00	*****	*****	.58	4.85	
FULV :FV	30.	101.	146.	.36	.18	870.53	*****	700.	870.17
530.	30.	167.	9136.	1.00	.00	.02	.57	4.81	

<<<<<THE ABOVE RESULTS REFLECT "NORMAL" (UNCONSTRICTED) FLOW>>>>

===135 CONVEYANCE RATIO OUTSIDE OF RECOMMENDED LIMITS.
 "APPR " KRATIO = 1.62

XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	
APPR :AS	60.	82.	228.	.15	.22	870.74	*****	700.	870.60
590.	60.	179.	14834.	1.00	.00	.00	.35	3.08	

<<<<<THE ABOVE RESULTS REFLECT "NORMAL" (UNCONSTRICTED) FLOW>>>>

<<<<<RESULTS REFLECTING THE CONSTRICTED FLOW FOLLOW>>>>

XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	
BR1 :BR	30.	131.	70.	2.33	.38	871.56	869.17	700.	869.23
530.	30.	153.	4331.	1.49	.85	.00	1.19	10.04	

TYPE PPCD FLOW C P/A LSEL BLEN XLAB XRAB
 2. **** 1. .821 ***** 877.40 ***** ***** *****

XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	

APPR :AS	30.	26.	596.	.02	.07	873.32	869.05	700.	873.30
590.	33.	201.	50062.	1.00	1.69	.01	.11	1.17	

M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL
.774	.633	18268.	133.	155.	873.30

<<<<<END OF BRIDGE COMPUTATIONS>>>>>

1

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
P060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
P-55-074 MANN VALLEY CREEK
EXISTING CONDITIONS

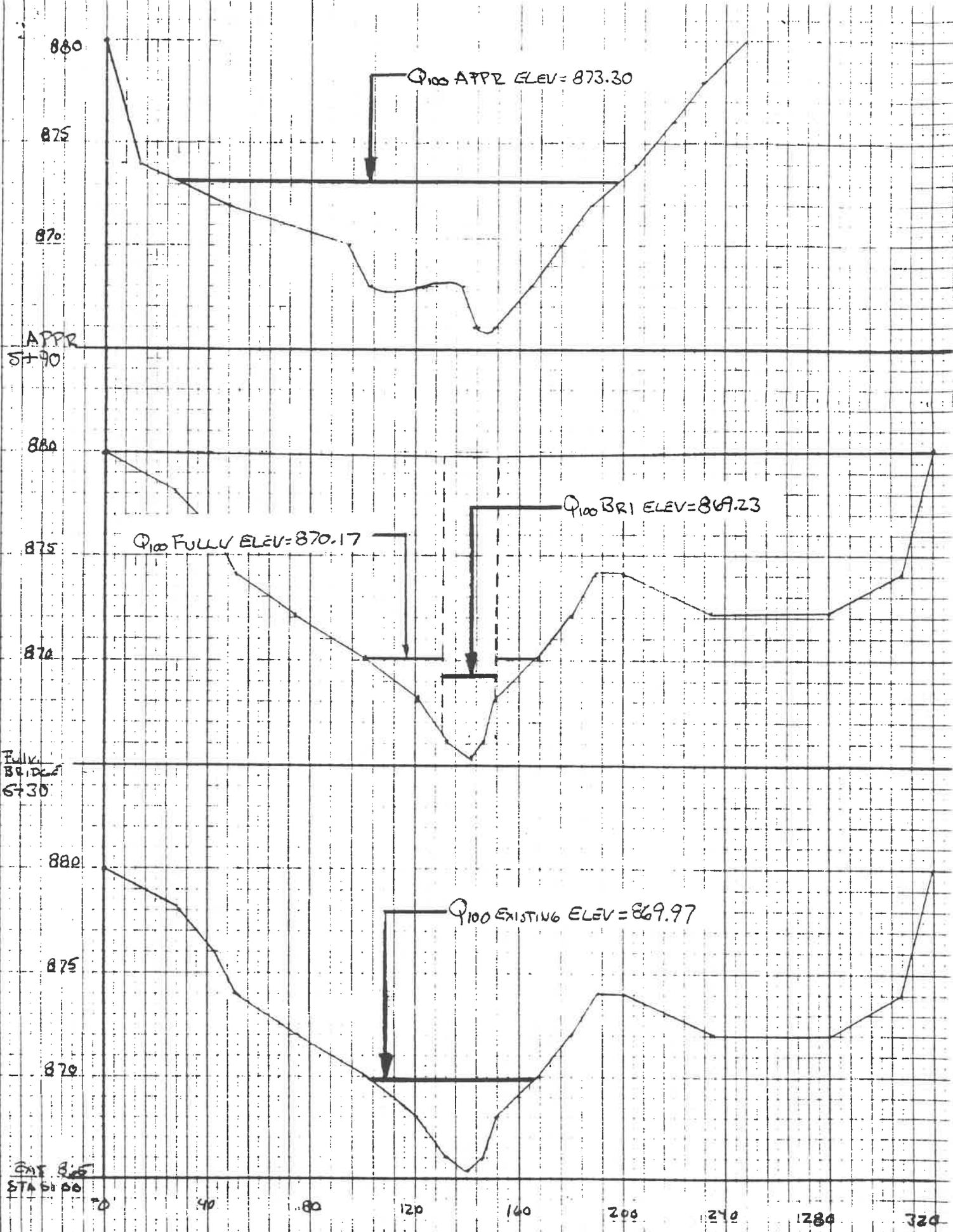
*** RUN DATE & TIME: 02-05-96 09:20

FIRST USER DEFINED TABLE.

XSID:CODE	SRD	Q	VEL	XSTW	YMIN	FR#	WSEL	EGL
EXIT :XS	500.	700.	4.85	65.	865.18	.58	869.97	870.34
FULV :FV	530.	700.	4.81	66.	865.36	.57	870.17	870.53
BR1 :BR	530.	700.	10.04	21.	865.36	1.19	869.23	871.56
APPR :AS	590.	700.	1.17	175.	865.72	.11	873.30	873.32

ER

NORMAL END OF WSPRO EXECUTION.



Q_{100} EXISTING CONDITIONS
 P-55-74 OLD

T1 CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
T2 P-55-149 MANN VALLEY CREEK
-T3 PROPOSED 42FT 28" "I" GIRDER
*
*

J1 1

*
*
J3 SDR Q VEL XSTW YMIN FR# WSEL EGL
6 5 13 28 23 14 3 7

*
*
Q 100YR.
700

JK 0.006

* X-SECTION STARTS AT STA 8+50 (ON PLAN AND PROFILE)
XS EXIT 500
JR 0,880 28,878.00 44,876 53,874 75,872 101,870
GR 121,868 133,866 140,865.18 146,866 152,868
GR 167,870 180,872 190,874 205,874 234,872 280,872 320,880
JA

MANNINGS N
N 0.040

IS FULV 530 * * * 0.006
* X-SECTION AT BRIDGE

BR BR1 530 876.55
BL 0 42.2 138 142
BD 3.43 880
CD 3 40 3 880
AB 1.5
J 0.045
*

AS APPR 590
GR 0,880 13,874 50,872 95,870 103,868 138,868
GR 143,866 145,865.72 148,866 165,868 175,870
JR 188,872 208,874 220,876 232,878 240,880
SA

* MANNINGS N
J 0.040

EX
TR

1
WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURV
P060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

*** RUN DATE & TIME: 02-05-96 09:32

T1 CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
T2 P-55-149 MANN VALLEY CREEK
T3 PROPOSED 42FT 28" "I" GIRDER
*
*

J1 1

J1 RECORD PARAMETERS:
DELTAY = 1.00 YTOL = .02 QTOL = .02 FNTEST = .80 IHFNOJ =

*
* SDR Q VEL XSTW YMIN FR# WSEL EGL
J3 6 5 13 28 23 14 3 7
*
*

* 100YR.
Q 700

*** Q-DATA FOR SEC-ID, ISEQ = 1

* SK 0.006
*

* X-SECTION STARTS AT STA 8+50 (ON PLAN AND PROFILE)

1
WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURV
P060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
P-55-149 MANN VALLEY CREEK
PROPOSED 42FT 28" "I" GIRDER

*** RUN DATE & TIME: 02-05-96 09:32

*** START PROCESSING CROSS SECTION - "EXIT "

XS EXIT 500
GR 0,880 28,878.00 44,876 53,874 75,872 101,870
GR 121,868 133,866 140,865.18 146,866 152,868
GR 167,870 180,872 190,874 205,874 234,872 280,872 320,880
SA

* MANNINGS N
N 0.040
*

*** FINISH PROCESSING CROSS SECTION - "EXIT "

*** CROSS SECTION "EXIT " WRITTEN TO DISK, RECORD NO. = 1

--- DATA SUMMARY FOR SECID "EXIT " AT SRD = 500. ERR-CODE = 1

SKEW IHFNO VSLOPE EK CK
.0 0. ***** .50 .00

X-Y COORDINATE PAIRS (NGP = 18):

X	Y	X	Y	X	Y	X	Y
.0	880.00	28.0	878.00	44.0	876.00	53.0	874

75.0	872.00	101.0	870.00	121.0	868.00	133.0	866.00
140.0	865.18	146.0	866.00	152.0	868.00	167.0	870.00
180.0	872.00	190.0	874.00	205.0	874.00	234.0	872.00
280.0	872.00	320.0	880.00				

X-Y MAX-MIN POINTS:

XMIN	Y	X	YMIN	XMAX	Y	X	YMAX
.0	880.00	140.0	865.18	320.0	880.00	.0	880.00

ROUGHNESS COEFFICIENTS (NSA = 1):
.040

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
P060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
P-55-149 MANN VALLEY CREEK
PROPOSED 42FT 28" "I" GIRDER
*** RUN DATE & TIME: 02-05-96 09:32

*** START PROCESSING CROSS SECTION - "FULV "

XS FULV 530 * * * 0.006
* X-SECTION AT BRIDGE

*** FINISH PROCESSING CROSS SECTION - "FULV "

*** NO ROUGHNESS DATA INPUT, WILL PROPAGATE FROM PREVIOUS CROSS SECTION.
*** CROSS SECTION "FULV " WRITTEN TO DISK, RECORD NO. = 2

--- DATA SUMMARY FOR SECID "FULV " AT SRD = 530. ERR-CODE = 0

SKEW	IHFNO	VSLOPE	EK	CK
.0	0.	.0060	.50	.00

X-Y COORDINATE PAIRS (NGP = 18):

X	Y	X	Y	X	Y	X	Y
.0	880.18	28.0	878.18	44.0	876.18	53.0	874.18
75.0	872.18	101.0	870.18	121.0	868.18	133.0	866.18
140.0	865.36	146.0	866.18	152.0	868.18	167.0	870.18
180.0	872.18	190.0	874.18	205.0	874.18	234.0	872.18
280.0	872.18	320.0	880.18				

X-Y MAX-MIN POINTS:

XMIN	Y	X	YMIN	XMAX	Y	X	YMAX
.0	880.18	140.0	865.36	320.0	880.18	.0	880.18

ROUGHNESS COEFFICIENTS (NSA = 1):
.040

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
P060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
P-55-149 MANN VALLEY CREEK
PROPOSED 42FT 28" "I" GIRDER
*** RUN DATE & TIME: 02-05-96 09:32

*** START PROCESSING CROSS SECTION - "BR1 "

BR BR1 530 876.55

BL 0 42.2 138 142
BD 3.43 880
CD 3 40 3 880
AB 1.5
N 0.045
*
*

*** FINISH PROCESSING CROSS SECTION - "BR1 "
*** CROSS SECTION "BR1 " WRITTEN TO DISK, RECORD NO. = 3

--- DATA SUMMARY FOR SECID "BR1 " AT SRD = 530. ERR-CODE = 0

SKEW IHFNO VSLOPE EK CK
.0 0. .0060 .50 .00

X-Y COORDINATE PAIRS (NGP = 6):

X	Y	X	Y	X	Y	X	Y
118.9	876.57	134.8	865.97	140.0	865.36	145.4	866.0
161.1	876.57	118.9	876.57				

X-Y MAX-MIN POINTS:

XMIN	Y	X	YMIN	XMAX	Y	X	YMAX
118.9	876.57	140.0	865.36	161.1	876.57	118.9	876.57

ROUGHNESS COEFFICIENTS (NSA = 1):
.045

BRIDGE PARAMETERS:

BRTYPE	BRWDTH	LSEL	USERCD	EMBSS	EMBELV	ABSLPL	ABSLPR
3	40.0	876.55	*****	3.00	880.00	1.50	*****

DESIGN DATA: BRLEN LOCOPT XCONLT XCONRT
42.2 0. 138. 142.

GIRDEP BDELEV BDSLP BDSTA
3.43 880.00 ***** *****

PIER DATA: NPW = 0 PPCD = **

1
WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
P060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
P-55-149 MANN VALLEY CREEK
PROPOSED 42FT 28" "I" GIRDER

*** RUN DATE & TIME: 02-05-96 09:32

*** START PROCESSING CROSS SECTION - "APPR "

AS APPR 590
GR 0,880 13,874 50,872 95,870 103,868 138,868
GR 143,866 145,865.72 148,866 165,868 175,870
GR 188,872 208,874 220,876 232,878 240,880
SA
* MANNINGS N
N 0.040
*
EX

BR1 :BR 30. 129. 70. 1.62 ***** 871.66 870.04 700804
 530. 30. 151. 4595. 1.04 ***** ***** 1.02 9.9

TYPE PPCD FLOW C P/A LSEL BLEN XLAB XRAB
 3. **** 1. .980 ***** 876.55 42. 135. 145.

XSID:CODE SRDL LEW AREA VHD HF EGL CRWS QSEL
 SRD FLEN REW K ALPH HO ERR FR# VL

APPR :AS 20. 30. 561. .02 .05 873.12 869.05 700809
 590. 23. 199. 46220. 1.00 1.41 .00 .12 1.2

M(G) M(K) KQ XLKQ XRKQ OTEL
 .764 .655 15956. 135. 154. 873.09

<<<<<END OF BRIDGE COMPUTATIONS>>>>

1
 WSPRO
 P060188

FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURV
 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
 (Input modified to free format by GKY&A 01/92)

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
 P-55-149 MANN VALLEY CREEK
 PROPOSED 42FT 28" "I" GIRDER

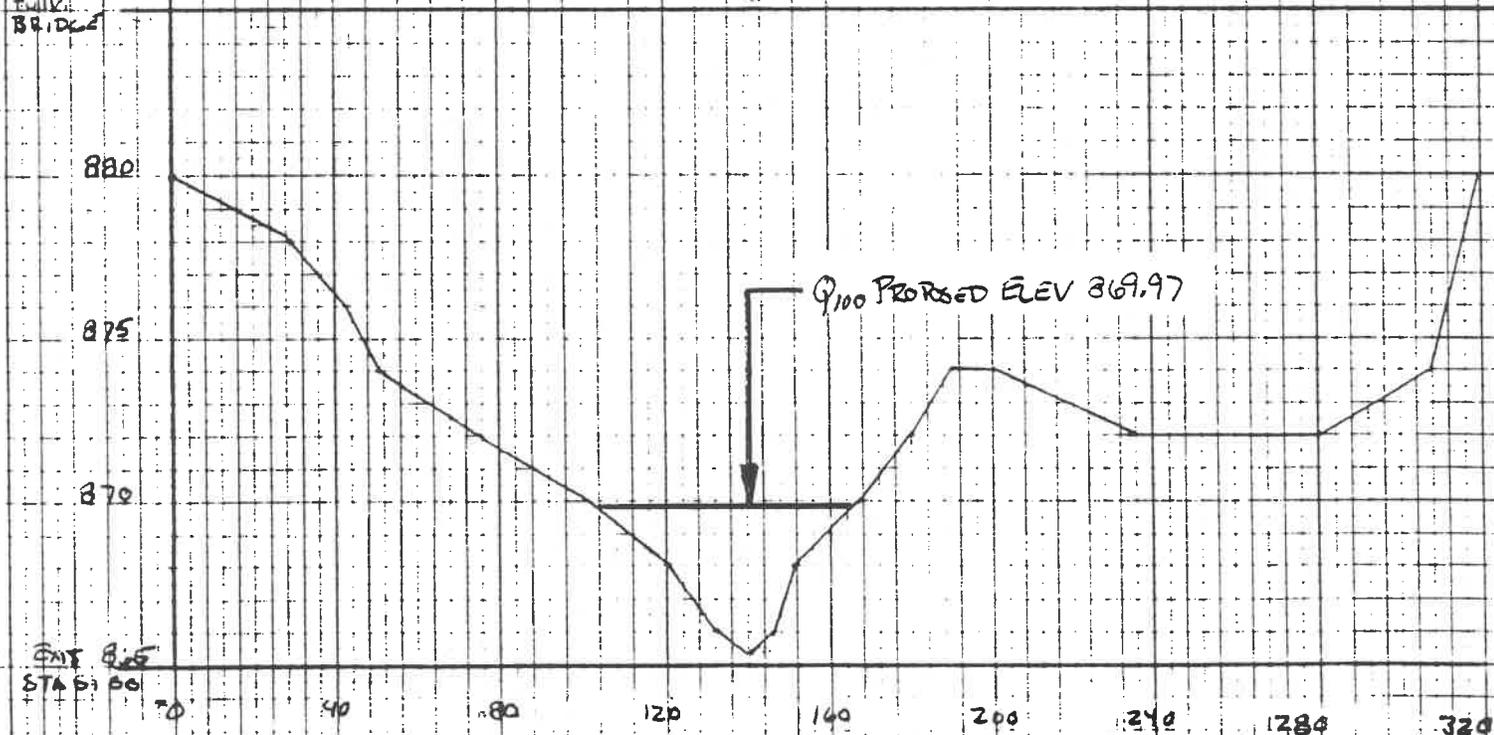
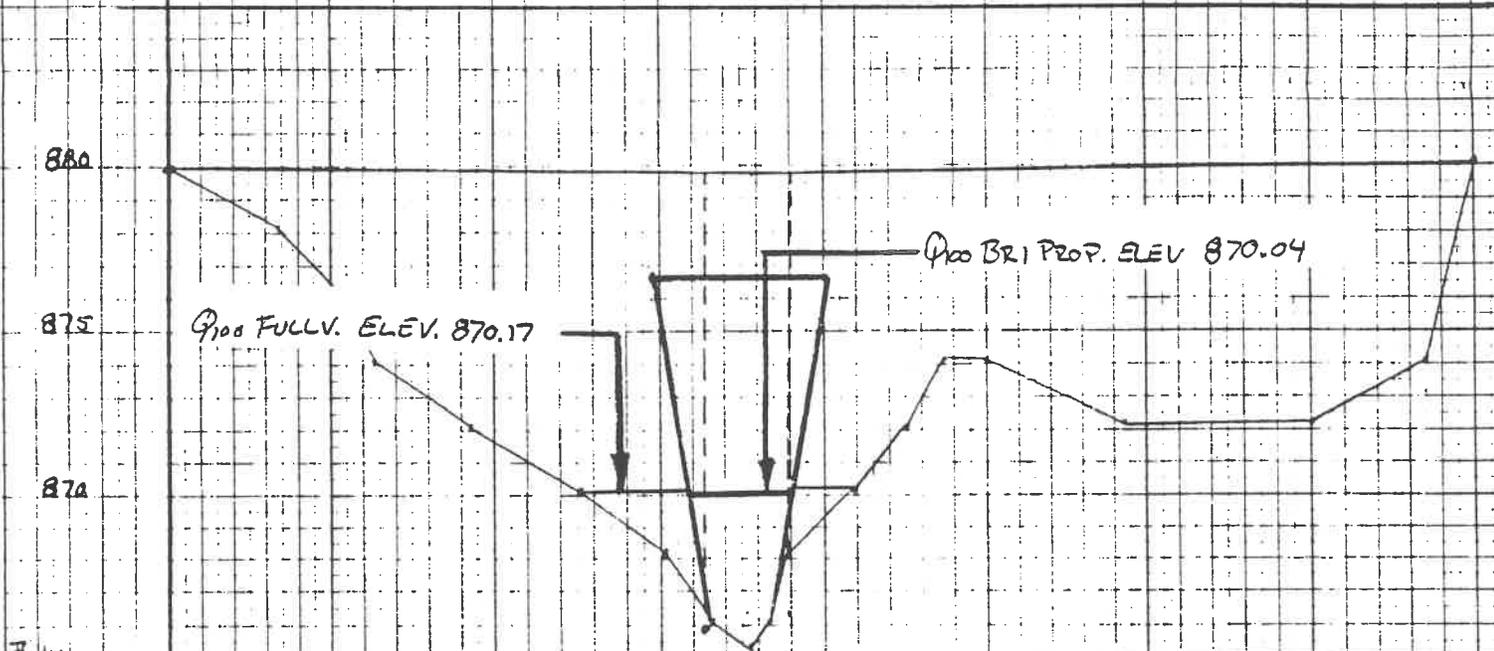
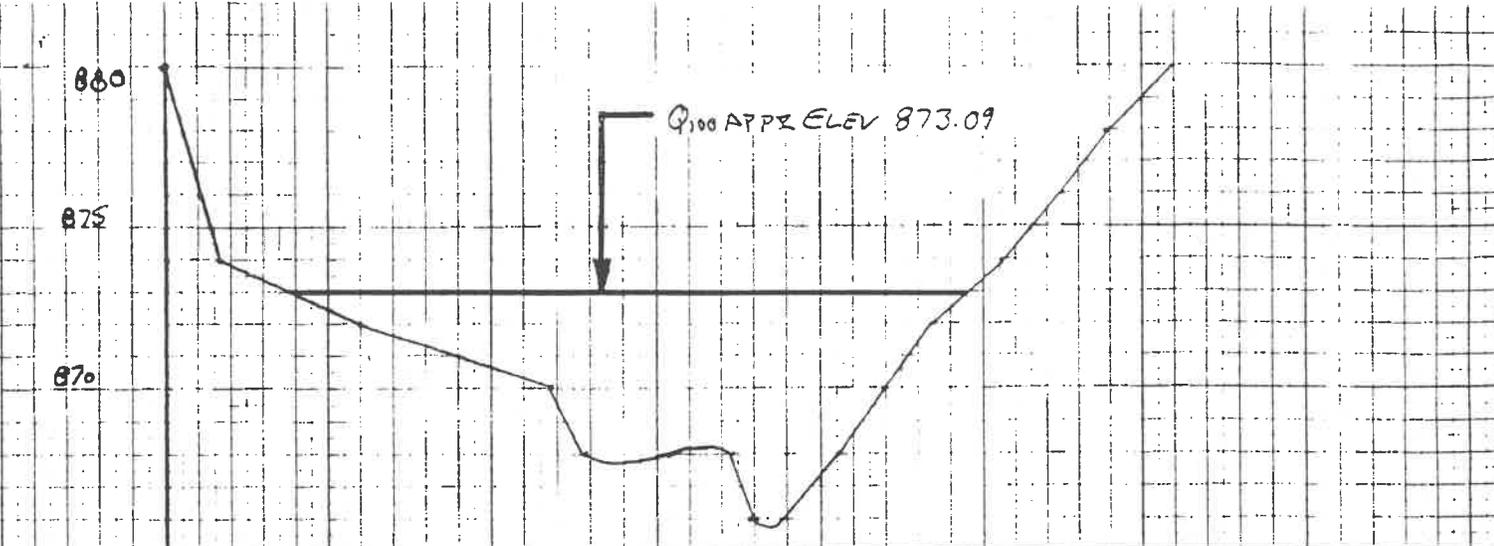
*** RUN DATE & TIME: 02-05-96 09:32

FIRST USER DEFINED TABLE.

XSID:CODE	SRD	Q	VEL	XSTW	YMIN	FR#	WSEL	IL
EXIT :XS	500.	700.	4.85	65.	865.18	.58	869.97	04
FULV :FV	530.	700.	4.81	66.	865.36	.57	870.17	03
BR1 :BR	530.	700.	9.99	23.	865.36	1.02	870.04	06
APPR :AS	590.	700.	1.25	169.	865.72	.12	873.09	02

ER

NORMAL END OF WSPRO EXECUTION.



ENT 8.5
STA 61 00

70 40 80 120 140 200 240 280 320

Q₁₀₀ PROPOSED BRIDGE
42' SINGLE SPAN 28" I-GIRDER
B-55-149

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
 P-55-149 MANN VALLEY CREEK
 PROPOSED 38FT 28" "I" GIRDER

1

SDR	Q	VEL	XSTW	YMIN	FR#	WSEL	EGL
6	5	13	28	23	14	3	7

100YR.
700

0.006

X-SECTION STARTS AT STA 8+50 (ON PLAN AND PROFILE)

EXIT 500

0,880	28,878.00	44,876	53,874	75,872	101,870
121,868	133,866	140,865.18	146,866	152,868	
167,870	180,872	190,874	205,874	234,872	280,872 320,880

MANNINGS N
0.040

FULV 530 * * * 0.006

X-SECTION AT BRIDGE

BR1	530	876.55
	38.2	138 142
	3.43	880
	3 40 3	880
	1.5	
	0.045	

APPR -590

0,880	13,874	50,872	95,870	103,868	138,868
143,866	145,865.72	148,866	165,868	175,870	
188,872	208,874	220,876	232,878	240,880	

MANNINGS N
0.040

EX
ER

1
 WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
 P060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
 (Input modified to free format by GKY&A 01/92)

*** RUN DATE & TIME: 02-05-96 09:43

T1 CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
 T2 P-55-149 MANN VALLEY CREEK
 T3 PROPOSED 38FT 28" "I" GIRDER
 *
 *
 J1 1

J1 RECORD PARAMETERS:
 DELTAY = 1.00 YTOL = .02 QTOL = .02 FNTEST = .80 IHFNOJ = -1

*
 * SDR Q VEL XSTW YMIN FR# WSEL EGL
 J3 6 5 13 28 23 14 3 7
 *
 *
 * 100YR.
 Q 700
 *** Q-DATA FOR SEC-ID, ISEQ = 1
 *
 SK 0.006
 *
 * X-SECTION STARTS AT STA 8+50 (ON PLAN AND PROFILE)

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
 P060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
 (Input modified to free format by GKY&A 01/92)

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
 P-55-149 MANN VALLEY CREEK
 PROPOSED 38FT 28" "I" GIRDER
 *** RUN DATE & TIME: 02-05-96 09:43

*** START PROCESSING CROSS SECTION - "EXIT "
 XS EXIT 500
 GR 0,880 28,878.00 44,876 53,874 75,872 101,870
 GR 121,868 133,866 140,865.18 146,866 152,868
 GR 167,870 180,872 190,874 205,874 234,872 280,872 320,880
 SA
 * MANNINGS N
 N 0.040
 *

*** FINISH PROCESSING CROSS SECTION - "EXIT "
 *** CROSS SECTION "EXIT " WRITTEN TO DISK, RECORD NO. = 1

--- DATA SUMMARY FOR SECID "EXIT " AT SRD = 500. ERR-CODE = 0

SKEW IHFNO VSLOPE EK CK
 .0 0. ***** .50 .00

X-Y COORDINATE PAIRS (NGP = 18):
 X Y X Y X Y X Y
 .0 880.00 28.0 878.00 44.0 876.00 53.0 874.00

75.0	872.00	101.0	870.00	121.0	868.00	133.0	866.0
140.0	865.18	146.0	866.00	152.0	868.00	167.0	870.0
180.0	872.00	190.0	874.00	205.0	874.00	234.0	872.0
280.0	872.00	320.0	880.00				

X-Y MAX-MIN POINTS:

XMIN	Y	X	YMIN	XMAX	Y	X	YMAX
.0	880.00	140.0	865.18	320.0	880.00	.0	880.0

ROUGHNESS COEFFICIENTS (NSA = 1):
.040

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
P060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
P-55-149 MANN VALLEY CREEK
PROPOSED 38FT 28" "I" GIRDER

*** RUN DATE & TIME: 02-05-96 09:43

*** START PROCESSING CROSS SECTION - "FULV "

XS FULV 530 * * * 0.006
* X-SECTION AT BRIDGE

*** FINISH PROCESSING CROSS SECTION - "FULV "

*** NO ROUGHNESS DATA INPUT, WILL PROPAGATE FROM PREVIOUS CROSS SECTION.
*** CROSS SECTION "FULV " WRITTEN TO DISK, RECORD NO. = 2

--- DATA SUMMARY FOR SECID "FULV " AT SRD = 530. ERR-CODE = 0

SKEW	IHFNO	VSLOPE	EK	CK
.0	0.	.0060	.50	.00

X-Y COORDINATE PAIRS (NGP = 18):

X	Y	X	Y	X	Y	X	Y
.0	880.18	28.0	878.18	44.0	876.18	53.0	874.B
75.0	872.18	101.0	870.18	121.0	868.18	133.0	866.B
140.0	865.36	146.0	866.18	152.0	868.18	167.0	870.B
180.0	872.18	190.0	874.18	205.0	874.18	234.0	872.B
280.0	872.18	320.0	880.18				

X-Y MAX-MIN POINTS:

XMIN	Y	X	YMIN	XMAX	Y	X	YMAX
.0	880.18	140.0	865.36	320.0	880.18	.0	880.B

ROUGHNESS COEFFICIENTS (NSA = 1):
.040

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
P060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
P-55-149 MANN VALLEY CREEK
PROPOSED 38FT 28" "I" GIRDER

*** RUN DATE & TIME: 02-05-96 09:43

*** START PROCESSING CROSS SECTION - "BR1 "

BR BR1 530 876.55

BL 0 38.2 138 142
BD 3.43 880
CD 3 40 3 880
AB 1.5
N 0.045
*
*

*** FINISH PROCESSING CROSS SECTION - "BR1 "
*** CROSS SECTION "BR1 " WRITTEN TO DISK, RECORD NO. = 3

--- DATA SUMMARY FOR SECID "BR1 " AT SRD = 530. ERR-CODE = 0

SKEW IHFNO VSLOPE EK CK
.0 0. .0060 .50 .00

X-Y COORDINATE PAIRS (NGP = 6):

X	Y	X	Y	X	Y	X	Y
120.9	876.57	137.2	865.69	140.0	865.36	142.8	865.75
159.1	876.57	120.9	876.57				

X-Y MAX-MIN POINTS:

XMIN	Y	X	YMIN	XMAX	Y	X	YMAX
120.9	876.57	140.0	865.36	159.1	876.57	120.9	876.57

ROUGHNESS COEFFICIENTS (NSA = 1):

.045

BRIDGE PARAMETERS:

BRTYPE	BRWIDTH	LSEL	USERCD	EMBSS	EMBELV	ABSLPL	ABSLPR
3	40.0	876.55	*****	3.00	880.00	1.50	*****

DESIGN DATA: BRLEN LOCOPT XCONLT XCONRT
38.2 0. 138. 142.

GIRDEP BDELEV BDSLP BDSTA
3.43 880.00 ***** *****

PIER DATA: NPW = 0 PPCD = **

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
P060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
P-55-149 MANN VALLEY CREEK
PROPOSED 38FT 28" "I" GIRDER

*** RUN DATE & TIME: 02-05-96 09:43

*** START PROCESSING CROSS SECTION - "APPR "

AS APPR 590
GR 0,880 13,874 50,872 95,870 103,868 138,868
GR 143,866 145,865.72 148,866 165,868 175,870
GR 188,872 208,874 220,876 232,878 240,880
SA

* MANNINGS N
N 0.040
*

EX

*** FINISH PROCESSING CROSS SECTION - "APPR "
 *** CROSS SECTION "APPR " WRITTEN TO DISK, RECORD NO. = 4

--- DATA SUMMARY FOR SECID "APPR " AT SRD = 590. ERR-CODE = 0

SKEW	IHFNO	VSLOPE	EK	CK
.0	0.	.0060	.50	.00

X-Y COORDINATE PAIRS (NGP = 16):

X	Y	X	Y	X	Y	X	Y
.0	880.00	13.0	874.00	50.0	872.00	95.0	870.00
103.0	868.00	138.0	868.00	143.0	866.00	145.0	865.72
148.0	866.00	165.0	868.00	175.0	870.00	188.0	872.00
208.0	874.00	220.0	876.00	232.0	878.00	240.0	880.00

X-Y MAX-MIN POINTS:

XMIN	Y	X	YMIN	XMAX	Y	X	YMAX
.0	880.00	145.0	865.72	240.0	880.00	.0	880.00

ROUGHNESS COEFFICIENTS (NSA = 1):
 .040

BRIDGE PROJECTION DATA: XREFLT XREFRT FDSTLT FDSTRT

+++ BEGINNING PROFILE CALCULATIONS -- 1

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
 P060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
 (Input modified to free format by GKY&A 01/92)

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
 P-55-149 MANN VALLEY CREEK
 PROPOSED 38FT 28" "I" GIRDER

*** RUN DATE & TIME: 02-05-96 09:43

XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	
EXIT :XS	*****	101.	144.	.37	*****	870.34	869.04	700.	869.97
500.	*****	167.	9033.	1.00	*****	*****	.58	4.85	

FULV :FV	30.	101.	146.	.36	.18	870.53	*****	700.	870.17
530.	30.	167.	9136.	1.00	.00	.02	.57	4.81	

<<<<<THE ABOVE RESULTS REFLECT "NORMAL" (UNCONSTRICTED) FLOW>>>>>

===135 CONVEYANCE RATIO OUTSIDE OF RECOMMENDED LIMITS.
 "APPR " KRATIO = 1.62

APPR :AS	60.	82.	228.	.15	.22	870.74	*****	700.	870.60
590.	60.	179.	14834.	1.00	.00	.00	.35	3.08	

<<<<<THE ABOVE RESULTS REFLECT "NORMAL" (UNCONSTRICTED) FLOW>>>>>

===285 CRITICAL WATER-SURFACE ELEVATION A S S U M E D !!!!!
 SECID "BR1 " Q,CRWS = 700. 870.79

<<<<<RESULTS REFLECTING THE CONSTRICTED FLOW FOLLOW>>>>>

XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	

BR1 :BR 30. 130. 68. 1.73 ***** 872.52 870.79 700. 870.79
 530. 30. 150. 4530. 1.06 ***** ***** 1.03 10.27

TYPE PPCD FLOW C P/A LSEL BLEN XLAB XRAB
 3. **** 1. .974 ***** 876.55 38. 137. 143.

XSID:CODE SRDL LEW AREA VHD HF EGL CRWS Q WSEL
 SRD FLEN REW K ALPH HO ERR FR# VEL

APPR :AS 20. 14. 713. .02 .04 873.95 869.05 700. 873.93
 590. 23. 207. 63101. 1.00 1.39 .00 .09 .98

M(G) M(K) KQ XLKQ XRKQ OTEL
 .805 .723 17506. 136. 154. 873.93

<<<<<END OF BRIDGE COMPUTATIONS>>>>>

WSPRO
 P060188

FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
 (Input modified to free format by GKY&A 01/92)

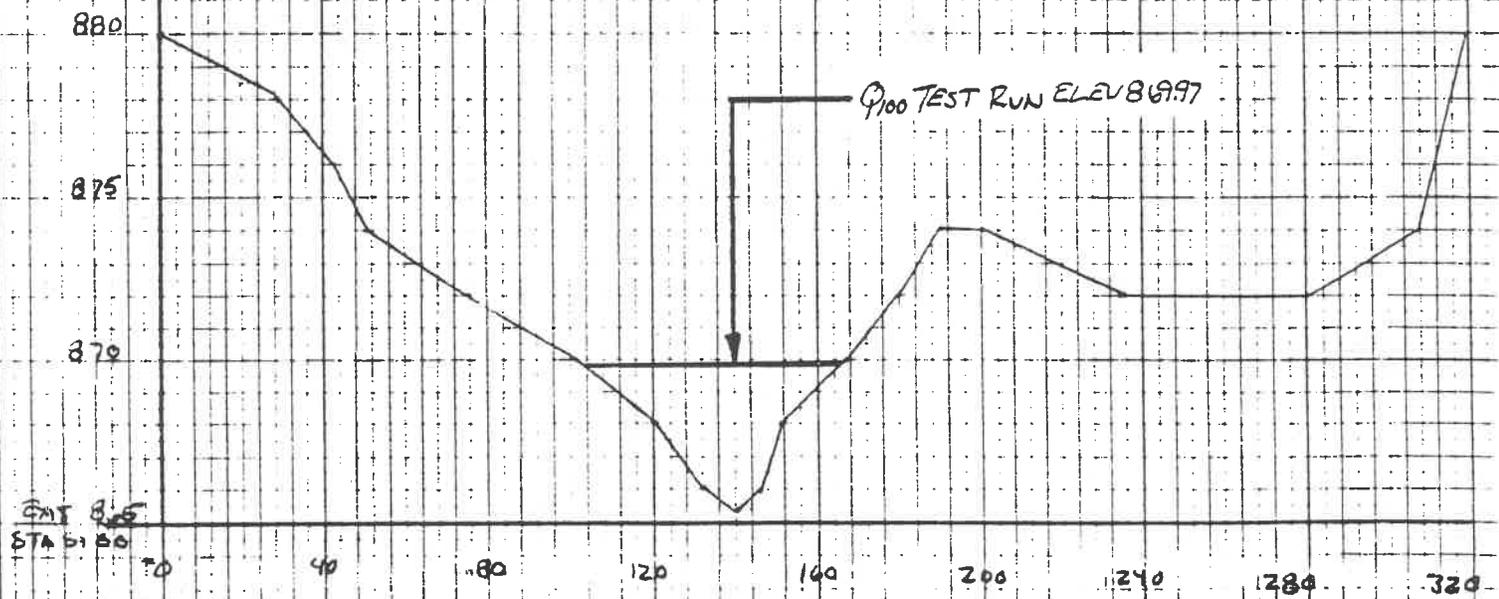
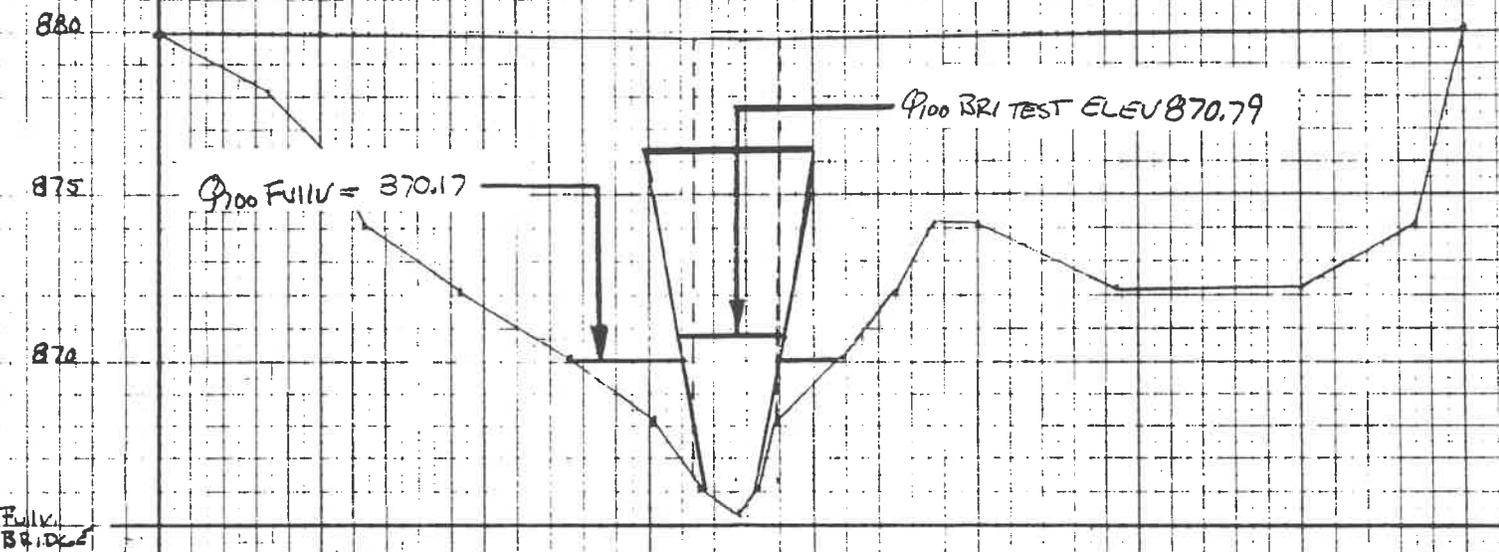
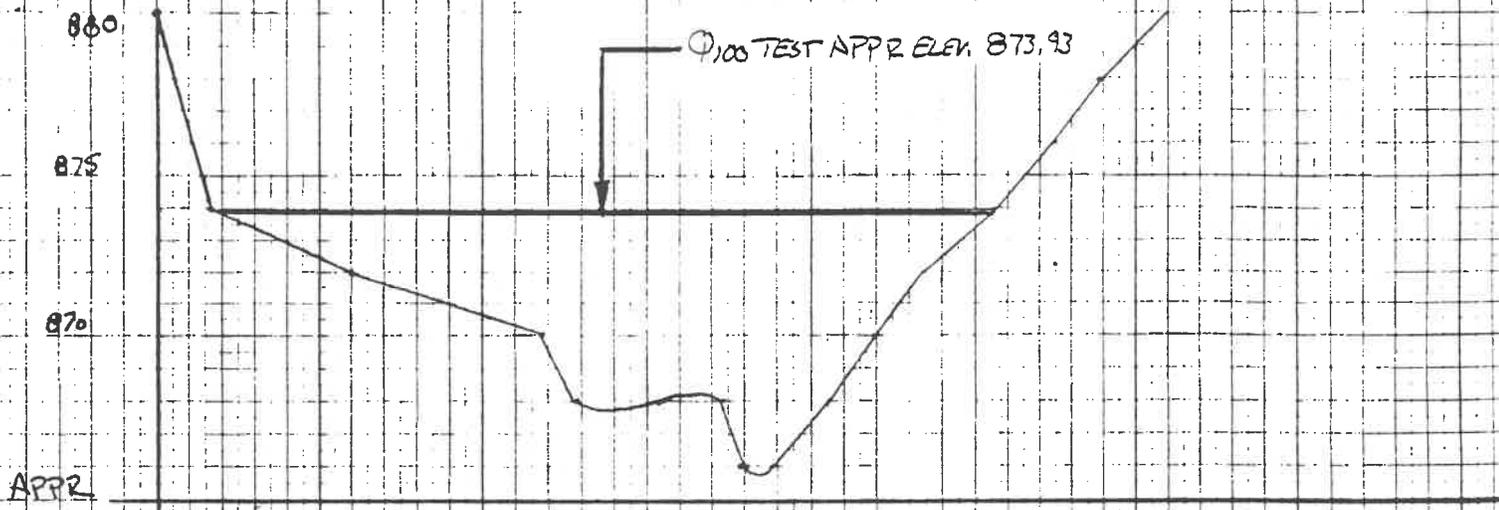
CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
 P-55-149 MANN VALLEY CREEK
 PROPOSED 38FT 28" "I" GIRDER

*** RUN DATE & TIME: 02-05-96 09:43

FIRST USER DEFINED TABLE.

XSID:CODE	SRD	Q	VEL	XSTW	YMIN	FR#	WSEL	EGL
EXIT :XS	500.	700.	4.85	65.	865.18	.58	869.97	870.34
FULV :FV	530.	700.	4.81	66.	865.36	.57	870.17	870.53
BR1 :BR	530.	700.	10.27	21.	865.36	1.03	870.79	872.52
APPR :AS	590.	700.	.98	193.	865.72	.09	873.93	873.95

NORMAL END OF WSPRO EXECUTION.



Q100 TEST RUN 38 FT
SINGLE SPAN I GIRDER

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
 P-55-149 MANN VALLEY CREEK
 PROPOSED 40FT FLAT SLAB

1

SDR	Q	VEL	XSTW	YMIN	FR#	WSEL	EGL
6	5	13	28	23	14	3	7

100YR.
700

0.006

X-SECTION STARTS AT STA 8+50 (ON PLAN AND PROFILE)
EXIT 500

0,880	28,878.00	44,876	53,874	75,872	101,870
121,868	133,866	140,865.18	146,866	152,868	
167,870	180,872	190,874	205,874	234,872	280,872 320,880

MANNINGS N
0.040

FULV 530 * * * 0.006
X-SECTION AT BRIDGE

BR1	530	876.7
	42.2	138 142
	2.32	880
	3	40 3 880
	1.5	
	0.045	

APPR 590

0,880	13,874	50,872	95,870	103,868	138,868
143,866	145,865.72	148,866	165,868	175,870	
188,872	208,874	220,876	232,878	240,880	

MANNINGS N
0.040

1

WSPRO
P060188

FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

*** RUN DATE & TIME: 02-05-96 09:27

T1 CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
T2 P-55-149 MANN VALLEY CREEK
T3 PROPOSED 40FT FLAT SLAB

*
*

J1 1

J1 RECORD PARAMETERS:

DELTAY = 1.00 YTOL = .02 QTOL = .02 FNTEST = .80 IHFNOJ = -1

*
* SDR Q VEL XSTW YMIN FR# WSEL EGL
J3 6 5 13 28 23 14 3 7

*
*

* 100YR.
Q 700

*** Q-DATA FOR SEC-ID, ISEQ = 1

*
SK

0.006

*
*

X-SECTION STARTS AT STA 8+50 (ON PLAN AND PROFILE)

WSPRO
P060188

FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
P-55-149 MANN VALLEY CREEK
PROPOSED 40FT FLAT SLAB

*** RUN DATE & TIME: 02-05-96 09:27

*** START PROCESSING CROSS SECTION - "EXIT "

XS EXIT 500
GR 0,880 28,878.00 44,876 53,874 75,872 101,870
GR 121,868 133,866 140,865.18 146,866 152,868
GR 167,870 180,872 190,874 205,874 234,872 280,872 320,880

SA
* MANNINGS N
N 0.040
*

*** FINISH PROCESSING CROSS SECTION - "EXIT "

*** CROSS SECTION "EXIT " WRITTEN TO DISK, RECORD NO. = 1

--- DATA SUMMARY FOR SECID "EXIT " AT SRD = 500. ERR-CODE = 0

SKEW IHFNO VSLOPE EK CK
.0 0. ***** .50 .00

X-Y COORDINATE PAIRS (NGP = 18):

X Y X Y X Y X Y
.0 880.00 28.0 878.00 44.0 876.00 53.0 874.00

75.0	872.00	101.0	870.00	121.0	868.00	133.0	866.00
140.0	865.18	146.0	866.00	152.0	868.00	167.0	870.00
180.0	872.00	190.0	874.00	205.0	874.00	234.0	872.00
280.0	872.00	320.0	880.00				

X-Y MAX-MIN POINTS:

XMIN	Y	X	YMIN	XMAX	Y	X	YMAX
.0	880.00	140.0	865.18	320.0	880.00	.0	880.00

ROUGHNESS COEFFICIENTS (NSA = 1):

.040

1

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
P060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
P-55-149 MANN VALLEY CREEK
PROPOSED 40FT FLAT SLAB

*** RUN DATE & TIME: 02-05-96 09:27

*** START PROCESSING CROSS SECTION - "FULV "

XS FULV 530 * * * 0.006

* X-SECTION AT BRIDGE

*** FINISH PROCESSING CROSS SECTION - "FULV "

*** NO ROUGHNESS DATA INPUT, WILL PROPAGATE FROM PREVIOUS CROSS SECTION.

*** CROSS SECTION "FULV " WRITTEN TO DISK, RECORD NO. = 2

--- DATA SUMMARY FOR SECID "FULV " AT SRD = 530. ERR-CODE = 0

SKEW	IHFNO	VSLOPE	EK	CK
.0	0.	.0060	.50	.00

X-Y COORDINATE PAIRS (NGP = 18):

X	Y	X	Y	X	Y	X	Y
.0	880.18	28.0	878.18	44.0	876.18	53.0	874.18
75.0	872.18	101.0	870.18	121.0	868.18	133.0	866.18
140.0	865.36	146.0	866.18	152.0	868.18	167.0	870.18
180.0	872.18	190.0	874.18	205.0	874.18	234.0	872.18
280.0	872.18	320.0	880.18				

X-Y MAX-MIN POINTS:

XMIN	Y	X	YMIN	XMAX	Y	X	YMAX
.0	880.18	140.0	865.36	320.0	880.18	.0	880.18

ROUGHNESS COEFFICIENTS (NSA = 1):

.040

1

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
P060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
P-55-149 MANN VALLEY CREEK
PROPOSED 40FT FLAT SLAB

*** RUN DATE & TIME: 02-05-96 09:27

*** START PROCESSING CROSS SECTION - "BR1 "

BR BR1 530 876.7

BL 0 42.2 138 142
BD 2.32 880
CD 3 40 3 880
AB 1.5
N 0.045
*
*

*** FINISH PROCESSING CROSS SECTION - "BR1 "
*** CROSS SECTION "BR1 " WRITTEN TO DISK, RECORD NO. = 3

--- DATA SUMMARY FOR SECID "BR1 " AT SRD = 530. ERR-CODE = 0

SKEW IHFNO VSLOPE EK CK
.0 0. .0060 .50 .00

X-Y COORDINATE PAIRS (NGP = 6):

X	Y	X	Y	X	Y	X	Y
118.9	877.68	136.8	865.74	140.0	865.36	143.3	865.81
161.1	877.68	118.9	877.68				

X-Y MAX-MIN POINTS:

XMIN	Y	X	YMIN	XMAX	Y	X	YMAX
118.9	877.68	140.0	865.36	161.1	877.68	118.9	877.61

ROUGHNESS COEFFICIENTS (NSA = 1):
.045

BRIDGE PARAMETERS:

BRTYPE	BRWDTH	LSEL	USERCD	EMBSS	EMBELV	ABSLPL	ABSLPR
3	40.0	876.70	*****	3.00	880.00	1.50	*****

DESIGN DATA: BRLEN LOCOPT XCONLT XCONRT
42.2 0. 138. 142.

GIRDEP BDELEV BDSLP BDSTA
2.32 880.00 ***** *****

PIER DATA: NPW = 0 PPCD = **

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
P060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
P-55-149 MANN VALLEY CREEK
PROPOSED 40FT FLAT SLAB

*** RUN DATE & TIME: 02-05-96 09:27

*** START PROCESSING CROSS SECTION - "APPR "

AS APPR 590
GR 0,880 13,874 50,872 95,870 103,868 138,868
GR 143,866 145,865.72 148,866 165,868 175,870
GR 188,872 208,874 220,876 232,878 240,880
SA

* MANNINGS N
N 0.040
*

EX

*** FINISH PROCESSING CROSS SECTION - "APPR "
 *** CROSS SECTION "APPR " WRITTEN TO DISK, RECORD NO. = 4

--- DATA SUMMARY FOR SECID "APPR " AT SRD = 590. ERR-CODE = 0

SKEW	IHFNO	VSLOPE	EK	CK
.0	0.	.0060	.50	.00

X-Y COORDINATE PAIRS (NGP = 16):

X	Y	X	Y	X	Y	X	Y
.0	880.00	138.0	874.00	50.0	872.00	95.0	870.00
103.0	868.00	138.0	868.00	143.0	866.00	145.0	865.72
148.0	866.00	165.0	868.00	175.0	870.00	188.0	872.00
208.0	874.00	220.0	876.00	232.0	878.00	240.0	880.00

X-Y MAX-MIN POINTS:

XMIN	Y	X	YMIN	XMAX	Y	X	YMAX
.0	880.00	145.0	865.72	240.0	880.00	.0	880.00

ROUGHNESS COEFFICIENTS (NSA = 1):
 .040

BRIDGE PROJECTION DATA: XREFLT XREFRT FDSTLT FDSTRT

+++ BEGINNING PROFILE CALCULATIONS -- 1

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
 P060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
 (Input modified to free format by GKY&A 01/92)

CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
 P-55-149 MANN VALLEY CREEK
 PROPOSED 40FT FLAT SLAB

*** RUN DATE & TIME: 02-05-96 09:27

XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	
EXIT :XS	*****	101.	144.	.37	*****	870.34	869.04	700.	869.97
500.	*****	167.	9033.	1.00	*****	*****	.58	4.85	
FULV :FV	30.	101.	146.	.36	.18	870.53	*****	700.	870.17
530.	30.	167.	9136.	1.00	.00	.02	.57	4.81	

<<<<<THE ABOVE RESULTS REFLECT "NORMAL" (UNCONSTRICTED) FLOW>>>>

===135 CONVEYANCE RATIO OUTSIDE OF RECOMMENDED LIMITS.
 "APPR " KRATIO = 1.62

APPR :AS	60.	82.	228.	.15	.22	870.74	*****	700.	870.50
590.	60.	179.	14834.	1.00	.00	.00	.35	3.08	

<<<<<THE ABOVE RESULTS REFLECT "NORMAL" (UNCONSTRICTED) FLOW>>>>

===285 CRITICAL WATER-SURFACE ELEVATION A S S U M E D !!!
 SECID "BR1 " Q,CRWS = 700. 870.63

<<<<<RESULTS REFLECTING THE CONSTRICTED FLOW FOLLOW>>>>

XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	

```

BR1  :BR      30.  129.    68.  1.72 ***** 872.35  870.63  700. 870.61
      530.    30.  151.   4534. 1.05 ***** ***** 1.03  10.21

TYPE PPCD FLOW      C    P/A    LSEL    BLEN    XLAB    XRAB
3. ****  1.    .975 ***** 876.70    42.    137.    143.

```

```

XSID:CODE  SRDL    LEW    AREA    VHD    HF    EGL    CRWS    |  WSEI
SRD        FLEN    REW      K    ALPH    HO    ERR    FR#    |  VEL

APPR :AS      20.    17.    684.    .02    .04  873.80  869.05  700. 873.71
      590.    23.   206.  59778. 1.00   1.40   .02    .09    1.01

M(G)    M(K)      KQ    XLKQ    XRKQ    OTEL
.798    .712    17107.  136.    154.    873.78

```

<<<<<END OF BRIDGE COMPUTATIONS>>>>

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1 WSPRO      FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVY
P060188     MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
              (Input modified to free format by GKY&A 01/92)

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CERNOHOUSE BRIDGE 96-M-1 ST.CROIX COUNTY CTH M
P-55-149 MANN VALLEY CREEK
PROPOSED 40FT FLAT SLAB

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*** RUN DATE & TIME: 02-05-96 09:27

FIRST USER DEFINED TABLE.

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XSID:CODE  SRD      Q      VEL    XSTW    YMIN    FR#    WSEL    EGL
EXIT :XS    500.    700.    4.85    65.    865.18    .58  869.97  870.34
FULV :FV    530.    700.    4.81    66.    865.36    .57  870.17  870.53
BR1  :BR    530.    700.   10.25    21.    865.36    1.03  870.63  872.35
APPR :AS    590.    700.    1.02   189.    865.72    .09  873.78  873.80

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ER

1 NORMAL END OF WSPRO EXECUTION.

River Falls Flood Insurance Study
Short Elliott Hendrickson, Inc.
Photograph date: 12-17-96



M
BRIDGE

Mann Valley looking upstream at Station 9576



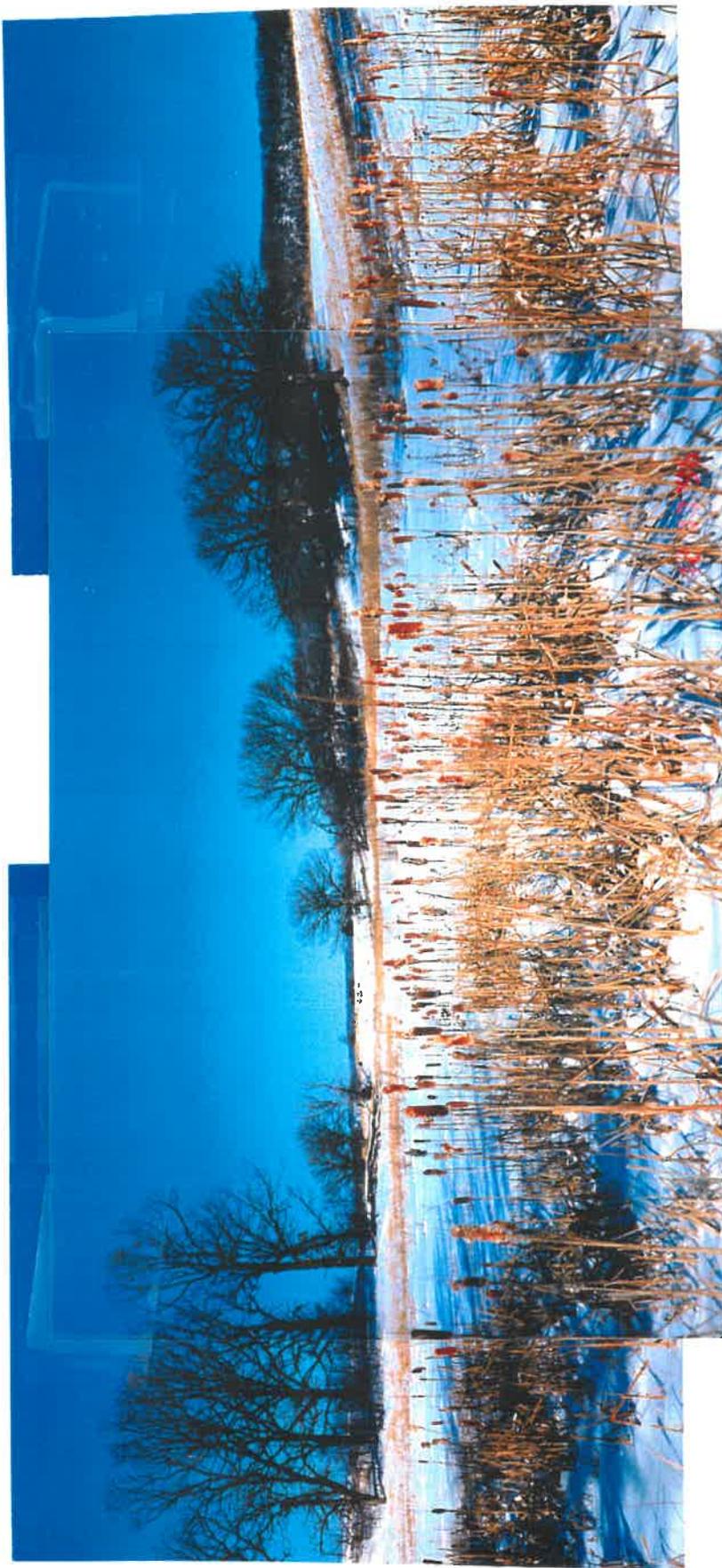
River Falls Flood Insurance Study
Short Elliott Hendrickson, Inc.
Photograph date: 12-17-96



Mann Valley looking upstream at Station 10125



River Falls Flood Insurance Study
Short Elliott Hendrickson, Inc.
Photograph date: 12-17-96



Mann Valley looking upstream at Station 10492



River Falls Flood Insurance Study
Short Elliott Hendrickson, Inc.
Photograph date: 12-17-96



Mann Valley looking upstream at Station 10912

